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22 **DEVELOPMENT OF COMPRESSION PULL-OFF TEST (CPOT) TO ASSESS BOND**
23 **STRENGTH OF BITUMEN**

24
25 Abdur Rahim ^{a,*}, Nick Thom ^b, Gordon Airey ^b

26 ^a *Department of Transportation Engineering and Management, University of Engineering and Technology, Lahore, Pakistan.*

27 ^b *Nottingham Transportation Engineering Centre, University of Nottingham, University Park, NG7 2RD, United Kingdom*

28 **Corresponding author. E-mail address: rahim@uet.edu.pk (A. Rahim)*
29

30 **ABSTRACT**

31 The quantification of moisture susceptibility has been a major concern for researchers as it
32 adversely affects the performance of asphalt pavements. Several methods have been
33 developed to assess bond strength using asphalt mixture in loose or compacted state. These
34 tests lack in their ability to study fundamental properties that the bond between bitumen and
35 aggregate. In this context, pull-off stub techniques have been developed such as pull-off stub
36 based tests and direct tension type tests. First group only measures the maximum pull-off
37 strength and second group has problems related to use of consistent binder film thickness and
38 operational problems in test itself.

39 This paper presents a new test to evaluate bond strength in an attempt to solve problems
40 associated with traditional pull-off techniques. This aim is achieved through review of existing
41 techniques, development of a direct tension test assembly and its evaluation, development of a
42 gap assembly and CPOT assembly. Key parameters for bitumen and mastics were evaluated.
43 The results show promising potential for use of this technique to study cohesive as well as
44 adhesive bond strength of binder.

45 Keywords: Pull-off test, asphalt, bitumen, adhesion, cohesion, bond strength tensile
46 strength, moisture damage, Compression Pull-Off Test, test development

47 **1 Introduction**

48 Binding material 'bitumen' is used in road construction all across the globe because of its good
49 adhesive, cohesive and waterproof characteristics. These bituminous pavements are
50 experiencing an ever increase in traffic load and its complexity. In addition to traffic loading,
51 there are environmental factors that adversely affect durability and integrity of asphalt mixture.
52 These include ageing and moisture damage as primary factors affecting durability of
53 pavements, provided that pavement is constructed according to specifications (Airey and Choi
54 2002). Ageing makes bitumen stiffer and brittle leading to its susceptibility of thermal cracking.
55 On the other hand, moisture damage result in deterioration of adhesive and cohesive bond in
56 asphalt mixtures.

57 Moisture damage is a complex phenomenon can be defined as "progressive functional
58 deterioration of a pavement mixture by loss of the adhesive bond between the bitumen and
59 aggregate surface (stripping) and/or loss of the cohesive resistance within the binder principally
60 from the action of water"(Kiggundu and Roberts 1988). The loss of bond strength due to water
61 damage lead to weaker pavement layer and makes it prone to deform under traffic loading
62 leading to deterioration (Airey and Choi 2002; Moraes, Velasquez, and Bahia 2011). The
63 cohesive failure occurs due to deformation under load at a distance from aggregate that is
64 beyond the influence of mechanical interlocking and surface molecular orientation
65 (Chaturabong and Bahia 2018). A cohesive failure mechanism can further to an adhesive failure
66 when the emulsification effects reach the aggregate surface (Fromm, 1974).

67 The five mechanisms which produce moisture damage, have been reported as; detachment,
68 displacement, spontaneous emulsification, pore pressure, and hydraulic scouring (Taylor A and
69 Khosla Paul 1983; Bagampadde, Isacson, and Kiggundu 2004) The later study has also
70 discussed microbial activity and osmosis as additional factors. There are four common
71 approaches/theories that explain the bond bitumen and aggregate; chemical reaction, surface

72 energy (surface change), molecular orientation, and mechanical adhesion (mechanistic
73 tenacity) (Shute et al. 1989; Bagampadde, Isacson, and Kiggundu 2004). Kringos et al. (2008)
74 have discussed three modes of moisture infiltration. The first being entry of water in connected
75 macro-pores through rainfall. Secondly, due to stationary moisture in the form of liquid or
76 vapour resides in the macro-pores. Third mode being the presence of water inside aggregate
77 before laying of the wearing course and inadequate drying of aggregate.

78 One of the earliest test to evaluate properties of bitumen included chew test. In this test,
79 builders tried to assess not only the consistency and but also stickiness of bitumen. Since then
80 several methods have been devised for measuring fracture of interfaces and adhesive joints. In
81 addition to these methods, several approaches has also been used to improve the bond
82 strength of bitumen and aggregate. This includes selection of suitable combination of binder
83 and aggregate, modification of bitumen (Baldi-Sevilla et al. 2017), improvement of mixing
84 techniques, and reduction of dust powder on surface of aggregate. However, there are
85 difficulties associated with these methods to improve bond between bitumen and aggregates
86 using these methods (Peng et al. 2018). In addition to this different antistripping additives such
87 as hydrated lime has been used to improve strength and reduce moisture susceptibility of
88 asphalt mixtures (Huang et al. 2005; Kim, Pinto, and Park 2012; Ameri, Kouchaki, and Roshani
89 2013; Zaidi 2018).

90 In discussion of moisture damage in asphalt mixture the tests classified in two categories; tests
91 conducted on loose and compacted mixtures, (Lottman et al. 1974). Several research has
92 summarised these moisture sensitivity tests (Terrel and Shute 1989; Terrel and Al-Swailmi
93 1994; Airey and Choi 2002; Bagampadde, Isacson, and Kiggundu 2004; Solaimanian et al.
94 2003). Test on loose mixture are empirical in nature and rely on visual inspection. On the other
95 hand, test on compacted mixtures are more fundamental in nature (Airey and Choi 2002).

96 Additionally, several studies has been performed to quantify bond strength based on fracture
97 parameters, surface energy, diffusion coefficients and adhesion (Kim, Pinto, and Park 2012).
98 Hitherto, there is no unified and standard fundamental test method for evaluation of bond
99 strength. (Moraes, Velasquez, and Bahia 2011) (Wang, Yi, and Feng 2014), (Rahim 2017) (Zhou
100 et al. 2018). This is because the amount of research performed to study tensile properties of
101 thin films of bitumen has been relatively little compared to other means of evaluating bitumen
102 properties (Chang 1994).

103 **2 Objectives**

104 This research aims to evaluate pull-off bond strength through development of a better, simple
105 and robust testing mechanism to solve problems associated with traditional pull-off testing
106 techniques. The test method developed to achieve this aim is termed the ‘Compression Pull-Off
107 Test (CPOT)’. Following objectives were defined for this research;

- 108 • Review of existing bond strength techniques
- 109 • Evaluation of direct tension test approach
- 110 • Development of mechanism to achieve required binder film thickness
- 111 • Development of compression pull-off test (CPOT)
- 112 • Evaluation of the key parameters for binder testing and validation of results

113 **3 Review of bond strength methods**

114 There has been numerous research efforts to select most appropriate methods to study
115 moisture sensitivity affected by loss of bond between bitumen and aggregate (Rice 1958;
116 Lottman et al. 1974; Terrel and Shute 1989; Airey and Choi 2002). The traditional methods on
117 loose and compacted mixtures such as Lottman’s procedure and its advanced modification i.e.
118 AASHTO T 283) are useful in comparative analysis of moisture susceptibilites of various HMA
119 mixtures. These methods, however, meausres bulk properties of mixtures and lack in focus on

120 fundamental material properties (Bhasin et al. 2006; Canestrari et al. 2010; Cho and Bahia
121 2010; Taylor, Hamed, and Nejad 2014). This creates the necessity to evaluate bond strength
122 using component characteristics tests which measure the fundamental properties. Additionally,
123 testing based on components characteristics is generally more economical (Kim, Pinto, and Park
124 2012). A fundamental test on binder and aggregate will give better understanding of moisture
125 sensitivity and its effect on the cohesive and adhesive bond of bitumen and aggregates
126 (Moraes, Velasquez, and Bahia 2011).

127 In a study, Jakarni (2012) has summarized some of the common tests used in science and
128 technology of adhesion to measure the adhesive bond strength of coatings of the composite
129 materials. These tests included peel test, pull-off test, double cantilever beam (DCB) test,
130 tapered double cantilever beam (TDCB) test, impact wedge peel (IWP) test and scratching of
131 thin films test. The review of experience in adhesive technology led Jakarni (2012) to
132 formulate a new pull-off testing technique to evaluate the bond strength.

133 In binder research, peel test has been used to quantify the adhesive strength (Blackman et al.
134 2013; Zhang et al. 2017; Zhou et al. 2018). A flexible thin peel arm is adhered on a substrate
135 with the use of adhesive material. A pull load is applied through peel arm at a constant speed
136 and specific angle and force to initiate and propagate peel fracture is measured. This peel force
137 is recorded as a function of the displacement to calculate fracture energy. In these studies, it
138 has been demonstrated that the peel test is a suitable method to determine the adhesive
139 fracture energy. In addition to this, there are numerous studies which emphasize the importance
140 of thin film binder to evaluate its response to pull-off loading (Kanitpong and Bahia 2005;
141 Jakarni 2012; A Copeland et al. 2006; Audrey Copeland, Youtcheff, and Shenoy 2007; Poulikakos
142 and Parti 2011; Alvarez, Ovalles, and Caro 2012; Harvey and Cebon 2003; Apeageyi, Grenfell,
143 and Airey 2014; Sultana 2014; Al-Haddad and Al-Khalid 2015; Rahim 2017; Abd, Al-Khalid, and
144 Akhtar 2018; Chaturabong and Bahia 2018)(Audrey Copeland, Youtcheff, and Shenoy 2007).

145 A commonly used pull-off stub type test is Pneumatic Adhesion Tensile Testing Instrument
146 (PATTI) to assess the moisture damage based on bond strength. Copeland (2007) modified the
147 procedure samples in PATTI to improve the control of the bitumen film thickness. (Santagata et
148 al. 2009) has also modified PATTI and reported on reliability and practicality to evaluate the
149 adhesion/cohesion properties of asphalt-aggregate system. One adaptability of PATTI is
150 Bitumen Bond Strength test (BBS) to evaluate pull-off strength (Kringos, N., Scarpas, A., and de
151 Bondt 2008; Moraes, Velasquez, and Bahia 2011; Zhou et al. 2018). BBS results are reported to
152 be reliable, repeatable and reproducible (Canestrari et al. 2010; Moraes, Velasquez, and Bahia
153 2011; Chaturabong and Bahia 2018; Mohammed et al. 2018). However, pull-off stub based
154 methods only measures the maximum pull-off strength and reports on description of coating
155 fracture. This pull-off stub approach has its limitation in a sense that rate of loading (in terms of
156 displacement) cannot be controlled (Zhang et al. 2017). In contrast another approach, Direct
157 Tension Test (DTT) methods have their advantage in measuring the pull-off load and elongation
158 (Rahim 2017; Abd, Al-Khalid, and Akhtar 2018).

159 There are several direct tension based studies to evaluate bond strength (Harvey and Cebon
160 2005; Jakarni 2012; Sultana 2014; Abd, Al-Khalid, and Akhtar 2018). In work with direct tension
161 tests, researchers have reported difficulties in preparing aggregate samples, achieving
162 consistent film thickness and perform tests itself (Abd, Al-Khalid, and Akhtar 2018). In DTT,
163 epoxy resin adhesive has been used to fix the aggregate plates with testing fixture (Peng et al.
164 2018). The use of these adhesive materials result in a slow process and require removal of
165 excessive adhesive from around the fixture. Another approach is to fix the aggregate plates in
166 testing moulds using screws. This fastening mechanism has operational problems associated to
167 it. The control of film thickness in specimens have been another concern, different approaches
168 and gap assemblies have been used (Jakarni 2012) . The use of Dynamic Shear Rheometer for
169 control of bitumen film at submicron level has also been reported (Zhang et al. 2017; Al-Haddad
170 and Al-Khalid 2015).

171 Surface Free Energy (SFE) is another useful technique to evaluate adhesion between aggregate
172 and binder. (Kringos, N., Scarpas, A., and de Bondt 2008) used surface energy approach and
173 combined direct tension test approach with numerical moisture diffusion analyses to
174 demonstrate that results consistent with expected field performance. The works highlights the
175 importance of importance of mechanical tests to assess bond strength. In addition, there has
176 been several attempts to correlate bond energy (from SFE) and total work of fracture (from
177 pull-off tests). The problem with SFE is that it is a thermodynamics based approach that do not
178 take into account energy dissipation during loading and unloading. Moreover, total work of
179 fracture (pull-off strength) is dependent on test geometry and testing conditions (Howson
180 2011).

181 The review of pull off techniques concludes with an importance of direct tension based
182 approach to evaluate the bond strength. There is, however, need to address the difficulties
183 associated with this test approach. Any such attempt must be effective to measure maximum
184 bond strength, rate of deformations corresponding to pull-off load and availability of fracture
185 surface to examine.

186 **4 Test materials**

187 The conventional properties of bitumen for this research are listed in Table 1. The work
188 included use of aluminium plates (35.1 mm diameter) as a control. This helped to ensure
189 substrate plates were parallel to achieve a uniform film thickness. The control material was also
190 convenient to use because of its high thermal conductivity to quickly cool down aluminium-
191 binder system. This ensured ready availability of gap assemble to prepare multiple specimen in
192 shorter interval of time. In addition, limestone (Ls) and granite (Gr) fillers (passing sieve 63
193 micron) were used to prepare mastics. The percentage of fillers was added as 40% by mass of
194 bitumen. These mastics were prepared by gradually mixing fillers in an oven heated bitumen

195 over a hot plate. In addition to these materials, limestone plates were used to validate the
196 results of CPOT.

197 **Table 1: Bulk mass properties of bitumen**

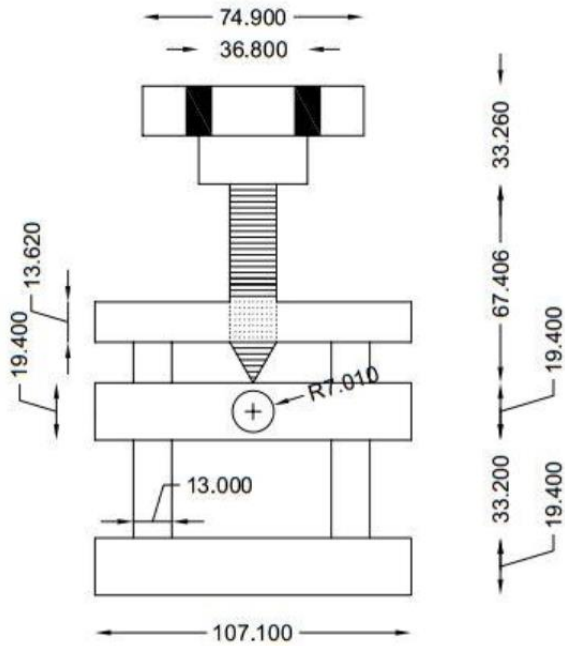
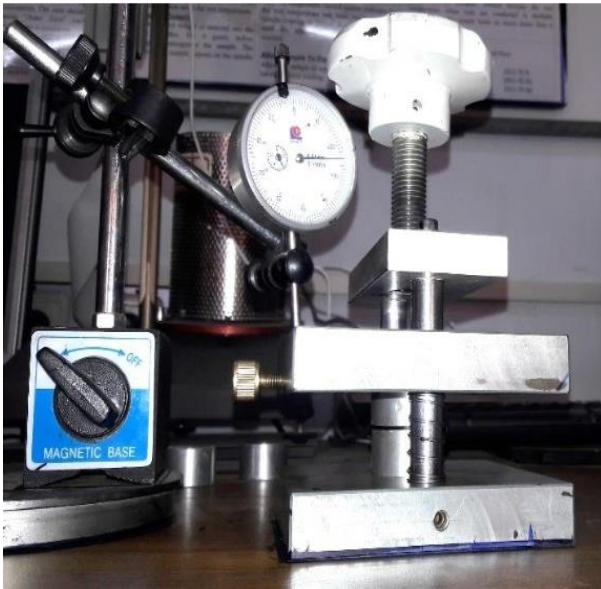
Bitumen	Source 1 Pen 60/70	Source 2 Pen 40/50	Source 2 Pen 60/70
Penetration Grade (dm)	64	47	69
Specific Gravity/Density	1.04	1	1.02
Softening Point (°C)	51	56	49.5

198 **5 Test methods**

199 A literature survey helped to formulate the basic requirements for a pull-off strength
200 evaluation. This required a direct and fundamental method to measure adhesive/cohesive bond
201 strength (DTT and CPOT). These methods were selected/devised on the basis of; direct
202 measurement of practical work of fracture, approach in which displacement can be controlled,
203 simplicity, practicality and cost effective to test on binder level. A gap assembly was also
204 devised to achieve required binder film thickness. In addition to these tests, RTFO was used for
205 short term ageing of bitumen and mastics.

206 **4.1 Development of gap assembly for film thickness**

207 A gap assembly was developed to achieve required film thickness of binder. In this assembly, a
208 compressive load was applied through a rotating disc. The resultant compression on binder was
209 measured with a deflection gauge (with precision 0.05mm and range 0-20mm). A 10° rotation
210 of the rotating disc compressed the bitumen layer to 50.56 micron. In order to achieve a
211 statistically consistent film thickness, the boundary of the whole assembly and position of the
212 needle was marked before testing. The compression device was able to apply a strong pressure
213 to uniformly compress even a penetration grade 40/60 at room temperature to layer of 0.5mm.
214 The lateral movement of substrate was prevented by providing grooves in the upper and
215 bottom plates of assembly.



216
217

Figure 1: The compression gap assembly

218 In specimen preparation, hot bitumen was poured onto an aluminium plate. A second plate was
 219 placed onto first plate to create a sandwiched specimen. A preconditioning of these plates at
 220 110 °C before pouring was identified to be a best practice. However, in case of plates
 221 conditioned at room a slight oven heating of sandwiched specimen was needed. The preheating
 222 before pouring helped to overcome hydrogen bond, p-p bonding and Van der Waals forces. The
 223 presences of these forces was expected to reduce the wetting of the substrate plates
 224 (Bagampadde et al., 2004). The sandwiched specimens were then compressed in gap assembly
 225 to achieve required thickness. After curing in gap assembly at room temperature, specimens
 226 were removed and left for additional curing/conditioning at room temperature for 24 hours.
 227 After conditioning period specimens were ready to test with direct tension test and CPOT.

228 **4.2 Direct tension test assembly**

229 In order to test contemporary direct tensile test (displacement controlled) approach, a direct
 230 tension assembly was fabricated. The modifications were made on the basis of joining

231 mechanism to connect with a 10 kN capacity Universal Testing Machine (UTM). The test
232 assembly is shown in Figure 2 (left). In tension testing, sandwiched aluminium plates were
233 gripped in the upper and lower moulds with three under-head screws. The top and bottom
234 extended arms of the moulds were fixed in the saw tooth grip of UTM as shown in Figure 2
235 (right). The testing was performed after adjusting the test parameters in software.



236
237

Figure 2: Direct pull-off testing assembly

238 In view of operational problems associated with DTT and variability of its result (section 5), a
239 better mechanism for testing was needed. The need for following improvements were
240 identified.

- 241 • The three-point grip (3 under-heads) was deemed insufficient in direct tension test.
242 During testing, aluminium plates were getting damaged due to drag and slip of plates. A
243 thorough gripping mechanism was needed.
- 244 • The surfaces of tested specimens were getting damaged, creating problem in analysing
245 the cohesive failure. This also added problem in observing phenomenon of cavitation.

- A consideration of authors' experience with pull off created need to conceptualise a better testing mechanism rather than adding further modifications to existing moulds.

4.3 Compression pull-off test assembly (CPOT)

The rationale of this test was derived from traditional compression tests performed on concrete and asphalt mixtures. The basic principle was thus formulated as “the load applied in compression of the UTM should generate a pull-off load on the specimen”. A conceptual diagram was prepared to translate this novel idea into reality (Figure 3, left). The idea was found to be feasible in terms of manufacturability. The assembly was designed and manufactured locally with ‘lean manufacturing’. This philosophy called for developing the product to a minimum functional level then improving the design according to needs (Shah and Ward 2003).

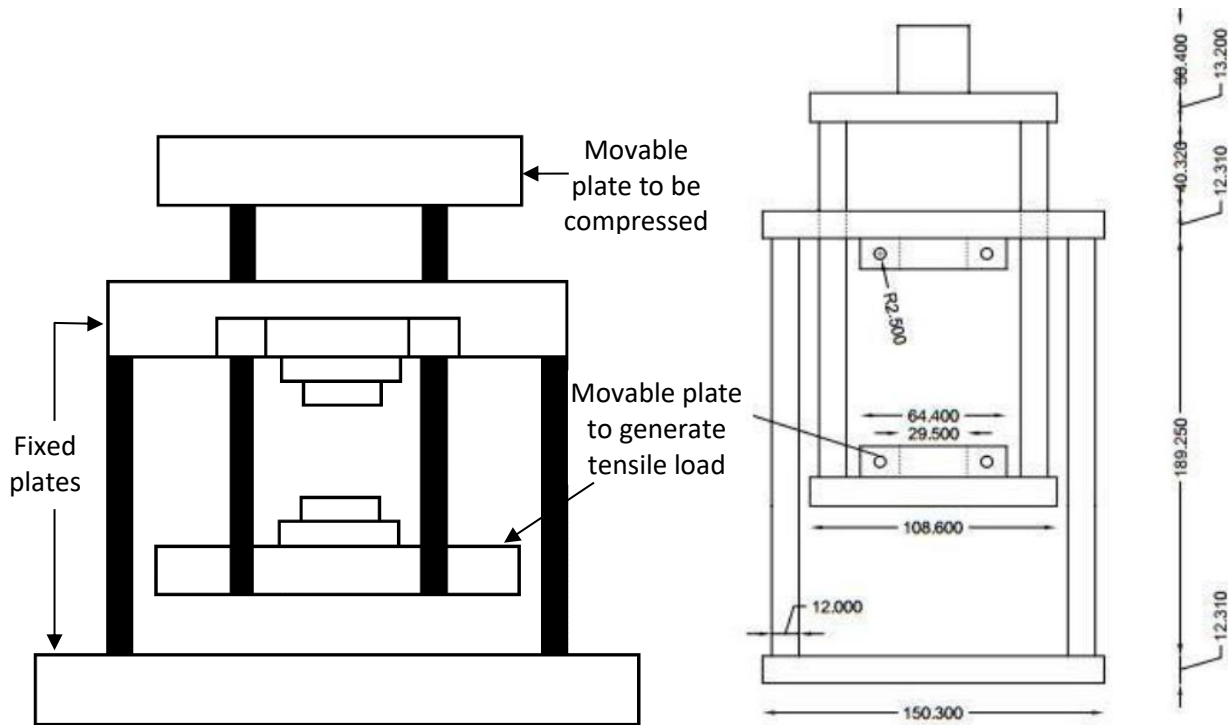
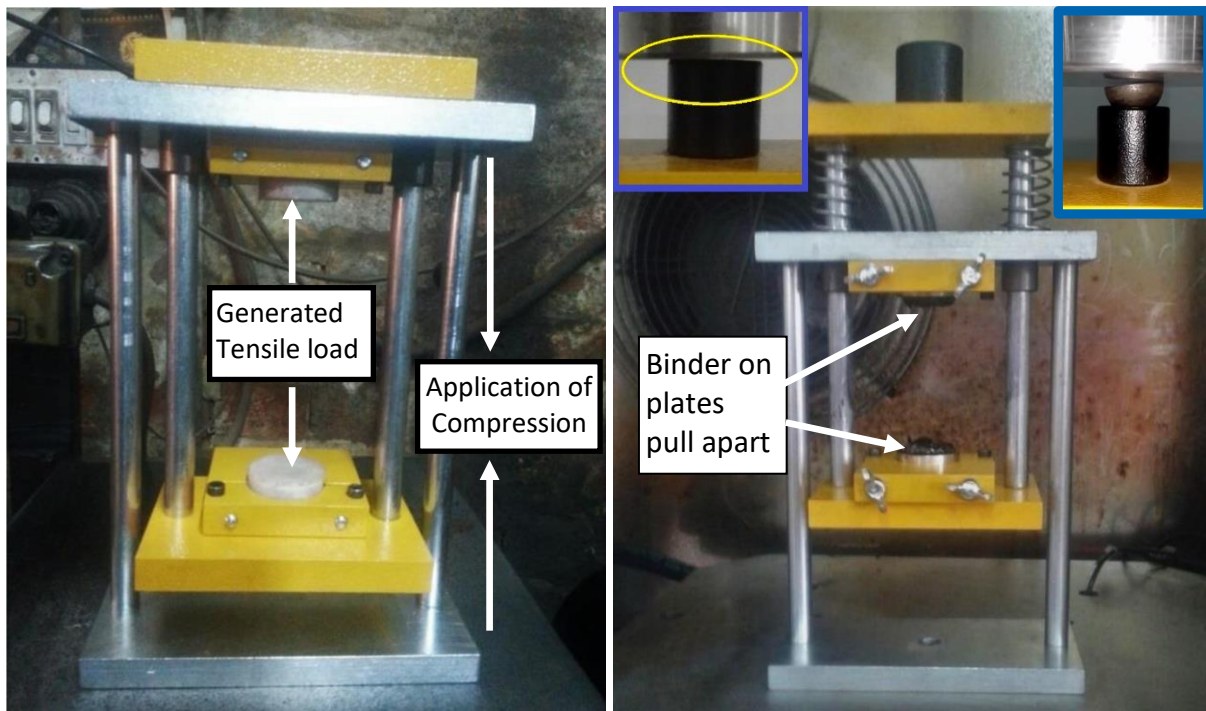


Figure 3: Conceptual diagram of compression device for pull-off testing

259 The Figure 4 (left) shows the first complete operational assembly. The sandwiched specimens
260 were fixed by means of two horizontal sliding plates. Circular groves in these plates to
261 thoroughly gripped the substrates. The screws in assembly were tighten to firmly squeeze the
262 substrate. This approach added a simplicity to design and ease of operation. Figure 4 (right),
263 shows the three further design improvements which were added as per need of trial testing. In
264 first modification two springs were added around the bars in upper moveable part to prevent it
265 from striking the fix part. In second modification, a shaft was introduced to movable part for
266 ease of centring and alignment. Thirdly, a half spheroid was introduced on the loading shaft for
267 the seating adjustments. A hole in the base of fix part was introduced for alignment purposes.
268 This assembly was named as 'Compression Pull-Off Test (CPOT) assembly'. This signified that
269 this assembly was able to take a compression load from an external source and translate it to a
270 uniform co-axial pull-off load on a binder-substrate system.



271
272

Figure 4: First prototype ad final design of CPOT assembly

273 The specimens prepared from gap assembly were inserted in the upper plate and tightened
274 with hand pressure. Lower plate was raised to grip the bottom aluminium substrate. After fixing
275 both ends of specimen, a slight pressure was applied with a small wrench to ensure firm grip. It
276 is important to mention that in entire testing phase no specimen broke during installing, fixing,
277 handling, and testing of assembly. This gripping mechanism seemed to have better tolerance
278 towards even slightly misaligned plates of sandwiched specimen.

279
280 In testing phase, the test parameters were defined in terms of loading direction, loading rate
281 (mm/min), specimen dimension and elongation limits etc. After this, a small seating load (few
282 Newtons) was applied to ensure proper contact. In some cases, specimens were discarded due
283 to application of accidental load application (higher than 100N). The final load was applied in
284 strain control mode and results were obtained in terms of load and elongation.

285

286 **5 Results and discussion**

287 The results were analysed in terms of mean pull-off strength (POS) and coefficient of various
288 (COV) for at least five replicates. Maximum pull-off strength of a specimen, expressed in MPa,
289 was calculated by dividing failure load in (F_n) by area of contact at surface i.e. $POS = 4 \frac{F_n}{\pi D^2}$,
290 where F_n is expressed in Newtons and mean diameter of substrate 'D' is expressed in (mm). The
291 acceptance of and rejection were based on observation as per some guidelines of ASTM:
292 D4541-17. The pull-off strength parameters evaluated for materials, instrumentation and key
293 test parameters.

294

295 **5.1 Repeatability and operator variability of gap assembly**

296

297 The accuracy of film thickness was checked with Vernier caliper for 36 specimens before and
298 after pouring of binder. These specimens were grouped according to plate condition. In cold

299 plate case, each specimen was prepared by pouring hot binder onto a clean control plate,
 300 which was preconditioned at room temperature. This plate condition was labelled as ‘cold’.
 301 The specimens in cold condition required slight heating in oven before introducing to gap
 302 assembly. In hot plate case, specimens were prepared by pouring hot bitumen onto preheated
 303 aluminium plates at 110 °C. This plate condition was labelled as ‘hot’. In comparison on method
 304 of pouring (Table 2), a good repeatability was achieved for pouring on hot aluminium plates.
 305 The operator 1 (experienced) achieved more repeatable results as compared to operator 2 with
 306 same method of pouring. This shows that the repeatability is dependent on method of
 307 specimen preparation and operator’s understanding of best laboratory practices.

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Table 2: Film thickness in gap assembly

Operator	Plate condition	Film Thickness (mm)							Replicates
		Max	Mean	Range	SD	COV (%)	Mean +2 SD	Mean -2 SD	
Operator 1	Cold	0.74	0.55	0.44	0.13	23.90	0.29	0.81	10
	Hot	0.63	0.50	0.21	0.06	12.09	0.38	0.63	10
Operator 2	Hot	0.58	0.45	0.33	0.10	21.92	0.25	0.65	16

310

311 In next stage, this effect of repeatability on these specimen was further evaluated with CPOT.
 312 Tests were performed with; 10mm/min rate of loading, 0.5mm film thickness, 15 hour dry/wet
 313 conditioning on untrimmed specimen. Two specimen were discarded for operator 1 in each
 314 case and six specimen for operator 2 were used for additional conditioning, the result of these
 315 specimens therefore are not included. The mean POS and COV values for these film thicknesses
 316 are shown in Table 3 to evaluate for repeatability.

317

318

Table 3: CPOT results for repeatability of gap assembly

Binder	POS	COV	Test condition	Replicates
Short term aged binder	1.17	13.54	Operator 1, dry 15 hours, cold	9
	1.25	8.40	Operator 1, wet 15 hours, hot	9
	1.53	14.80	Operator 2, dry 15 hours, hot	10

319 **Operator 1 and 2 for dry conditioning:** In Table 2 and Table 3, operator 1 achieved 100 μm
320 more film thickness for than operator. This should have led to a slightly lower POS theoretically.
321 However the actual difference is significantly less, possibly due to bad bonding because of cold
322 plate condition and non-uniformity of binder inside the plates.

323 **Comparison of dry and wet for hot plate condition:** Operator 2 should have achieved slightly
324 increased POS due to 50 μm decrease of film thickness. However, dry POS is significantly higher
325 as compared to wet conditioning. This suggests main cause of adverse effect is moisture
326 conditioning.

327 **Comparison for cold/hot and dry/wet for operator 1:** The wet conditioning should have
328 resulted in decrease of POS and a 50 μm film thickness difference should result in slight
329 increase of POS. The net effect should be a decrease, this is however not the case. The only
330 possible explanation is a bad bond achieved with cold plate condition.

331

332 The quality of results for good and bad bonding was checked through load-elongation curves.
333 The results of that gap assembly are repeatable for hot plate condition (already identified as a
334 best laboratory practice in section 5.1). The film thickness and CPOT tests were performed on
335 an aged (stiffer) binder. The results showed that repeatability was dependent on specimen
336 preparation method and operator's understanding of best laboratory practices. In further tests,
337 cold plate condition needed careful consideration for mastics and aged binder.

338 **5.2 Results and discussion on DTT results**

339 The sandwiched specimens prepared with gap assembly were tested with DTT moulds. In
340 testing on bitumen specimens were by pouring hot bitumen on a hot aluminium plate and
341 trimming the specimen after curing. For granite mastic, binder was prepared in cold plate
342 condition and tested under untrimmed condition. The results show a variability of 16.8% (Table
343 4) for virgin bitumen and 33.9% in case of granite mastic. This significant variability is caused by
344 the combined action of specimen preparation method as well as testing with DTT moulds itself.

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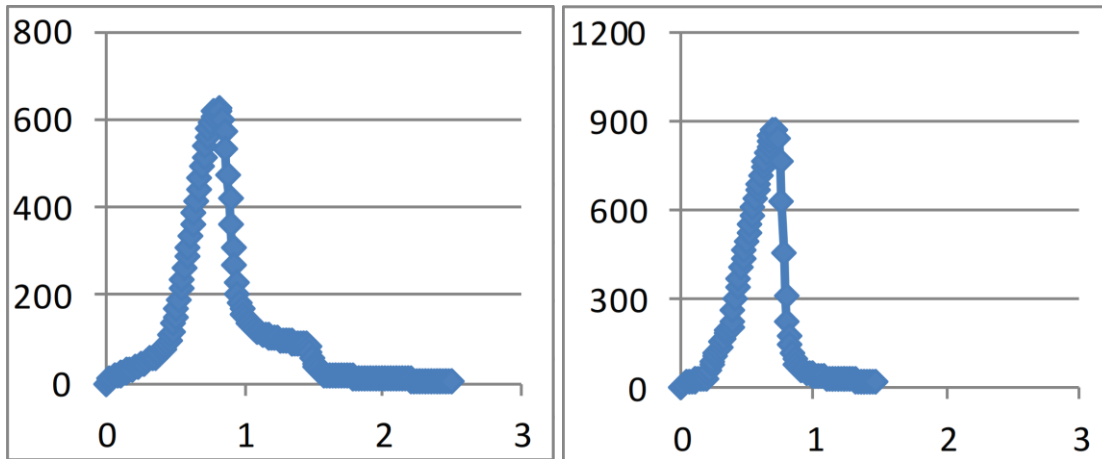
Table 4: Summary of test results using direct pull-off testing

Material	Pull-Off Strength (MPa)	COV (%)	Plate Condition	Replicates
Source 2 Pen 60/70	0.71	16.78	Hot	6
Gr (40%) in Source 1 Pen 40/60	1.01	33.86	Cold	6

347

348 Figure 5 shows ductile (left) and brittle (right) mode of failures for bitumen and mastic
349 respectively. This brittle failure indicates a sudden drop in load carrying capacity after failure
350 load. This brittle failure may have been cause by increased stiffness and bonding due to cold
351 conditions providing weakest plane of failure in specimen. The ductile mode was dominate for
352 virgin bitumen and is identified from the ability of the material to take load after failure in
353 cohesive mode. This indicate that bond failure starts within the molecules of the bitumen or
354 bitumen mastic interface due to nucleation of micro voids. The negative slope in the softening
355 portion is dependent on the degree of ductility of material (Poulikakos and Parti 2011).

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Figure 5: Brittle and ductile type failure using DTT

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The direct tension assembly was provided with double universal joint on the upper and lower moulds for seating and self-aligning. This modification was created in an attempt to increase the repeatability of test results. However this resulted into damage of failure surfaces. The DTT of this research as well as other techniques in this approach require several procedural steps. This lead to excessive handling and breaking of specimens. Also some specimen slipped out of moulds during test, leading to misleading results. In order to solve these issues a new design and approach necessary.

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5.3 Discussion on Results of CPOT

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In this test, the main objectives was to evaluate parameters related to testing mechanism and material to establish its usefulness. The parameters related to specimen were evaluated using direct pouring and silicon method of pouring. The constant test parameters included; film thickness (0.5mm), rate of loading (10mm/min), cold plate condition, 24 hours dry conditioning, and room temperature testing. Table 5 shows a decrease of pull-off strength and increased coefficient of variability with silicon pouring and trimming for bitumen in round 1. This was also found to be true for tests on 3 hours aged binder (round 2). However at this stage, POS include combined the effect of method of pouring and trimming of specimen. A third round of tests were performed on RTFO aged binder to exclude the effect of trimming. The bond strength

376 decreased by 20.6% only by changing the method of pouring. Silicon method of pouring gave
 377 lower POS with increased variability. The unaged binder showed only a 9.1% decrease of binder
 378 in bond strength. This difference of can be attributed to trimming and ageing. The aged binder
 379 shows increased bond strength with shift from ductile to brittle failure. The effects of silicon
 380 pouring and trimming seems to be contradictory with Dynamic Shear Rheometer testing in
 381 which former is a compulsory step and second is an alternative to direct pouring. This is
 382 attributed to different nature of two tests for quantification of tensile strength (pull-off
 383 strength) and shear strength respectively. In addition to this, different studies have used
 384 trimming of specimen in pull off testing (Sultana 2014; Al-Haddad and Al-Khalid 2015; Apeageyi,
 385 Grenfell, and Airey 2015; Abd, Al-Khalid, and Akhtar 2018). This study suggests effect of
 386 trimming on the results of pull-off strength. The coefficient of variability in all test groups
 387 indicate a good repeatability of CPOT results.

388 **Table 5: Tests to check repeatability of the test method**

Round	Material	Pull-Off Strength (MPa)	COV (%)	Variable Testing conditions	Replicates
Round 1	Source 2 Pen	0.70	9.67	Silicon, tr	5
	60/70	0.77	6.82	Direct, un	8
Round 2	3-hour oven aged Source 1	1.4	13.35	Silicon, tr	9
	Pen 40/50	1.45	11.04	Direct, un	8
Round 3	RTFO aged Source 1 Pen	1.00	11.63	Silicon, tr	7
	60/70	1.26	8.35	Direct, tr	9

389 In Table effect of decrease in film thickness have resulted in increase of pull-off bond strength.

390 The mode of failure in case of 0.5mm film thickness was cohesive while for 0.3mm film

391 thickness it changed from hybrid to adhesive. This is consistent with findings in literature on the
 392 effect of film thickness on pull-off bond strength (Marek and Herrin 1968; Chang 1994; Fond
 393 2001; Harvey and Cebon 2003, 2005; Poulidakos and Parti 2011; Jakarni 2012; Sultana 2014;
 394 Abd, Al-Khalid, and Akhtar 2018).

395 **Table 6: Effect of film thickness on pull-off bond strength**

Material	Mean POS (MPa)	COV (%)	Film Thickness (mm)	Failure Mechanism	Replicate
Source 2 Pen 60/70	1.29	20.97	0.3	Hybrid to adhesive	7
Source 2 Pen 60/70	0.71	16.78	0.5	Cohesive	6

396 The typical failure curves in ductile mode are presented in Figure 6 for comparison of DTT and
 397 CPOT. Firstly in both tests, a deviation from this is attributed to an anomaly in test specimens
 398 and test itself. These problem could arise due to issue in specimen preparation,
 399 curing/conditioning, non-uniform film thickness, non-homogenous mixing of filler particles and
 400 mainly slipping of plates during test. The load elongation curve is advantageous to both method
 401 in comparison to pull-off stub tests. In comparison to CPOT, direct tension test curve before
 402 peak load is different due to seating adjustments. The CPOT has already taken care of major
 403 seating adjustment during manual adjustment prior to start of test. The CPOT curve can be
 404 directly analysed while DTT curve need normalisation to study energy dissipation. Hitherto,
 405 authors have found no discussion in binder studies dealing with advantage due to compression
 406 approach. CPOT is useful in study of load elongation curves as compared to contemporary
 407 direct tension tests.

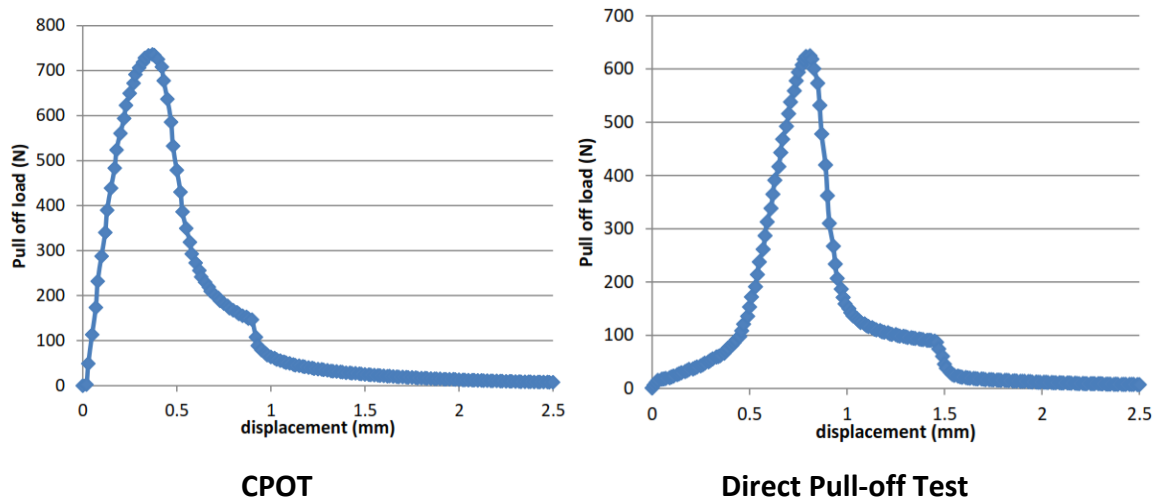


Figure 6: Typical ductile failure with CPOT and direct pull-off testing

5.4 Validation of CPOT results

The results were validate with RTFO aged limestone mastics and use of limestone aggregate plates. The conditioning time was limited to 24 hours as main of equilibrium uptake occurs during this conditioning period (Apeageyi, Grenfell, and Airey 2015). Table 5 shows strength of aged mastic has significantly decreased tested with moisture conditioning. The dominate type of failure in this case hybrid to adhesive. This is because mastics and aggregate plates allow easier access of water within the mass of binder and to the interface. The dry conditioned specimens have also shown tendency towards hybrid to adhesive failures in case of aggregate plates. This may have been caused by residual dust on aggregate plates, in addition to less control over plate surface as compared to control. This have increase chance of weak failure plane near the bitumen aggregate interface. The cohesive strength is only completely available if the interface bond between binder and aggregate is of good quality. The CPOT results have shown a promising results to evaluate moisture damage.

425

Table 5: CPOT results with aggregate plates and moisture conditioning

Material	Mean POS (MPa)	COV (%)	Conditioning	Plates	Replicate
Short term aged Ls	1.38	11.53	Dry	New	6
40% in Source 1	0.96	33.31	Dry	Used	5
Pen 40/50	0.81	16.15	Wet	New	6

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A second evaluation was made with re-use of cleaned aggregate plates. The results indicate a significant reduction in bond strength also with increased coefficient of variance. This is because of decrease in presence of activate bond sites on re-used aggregate surface. This may have been caused by presence of water due to insufficient drying or accumulation of oily components due adsorption of binder in first use.

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These findings in addition to discussion on previous results of CPOT have shown good repeatability and robustness of the method to test parameters related to material, ageing and moisture conditioning. This test is successful in quantifying the bond strength of bitumen involving fillers, effect of ageing, and moisture conditioning.

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6 CPOT results comparison with developed DTT

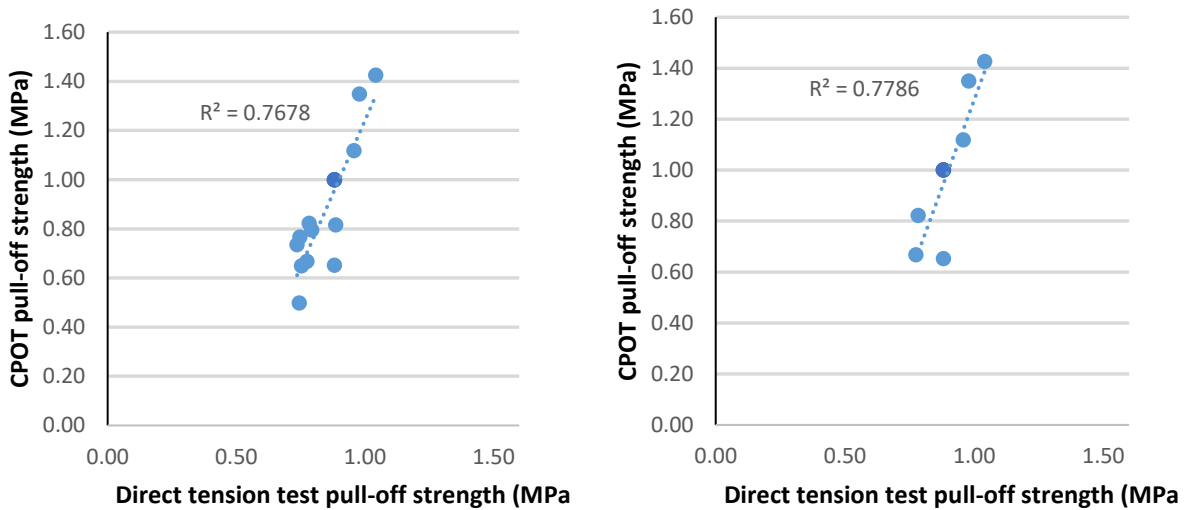
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The results of the two test assemblies are presented in Table 6. It is evident from the variability of the CPOT results that this method is repeatable and effective in evaluating bond strengths. The variability of the Direct Pull-Off Test developed in this study is significantly higher than CPOT. This is in the case when CPOT specimens were poured onto a cold plate (condition selected to cause variability in results). A correlational analysis was performed between two tests as shown in Figure 7. The test shows a medium correlation between the two tests. CPOT seems to measure higher bond strength then DTT approach used in this research with limited tests results. This quantitative comparison needs further evaluation of DTT results.

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Table 6: CPOT Vs Direct Pull-Off Testing Assembly

Method	Material	POS (MPa)	COV (%)	Condition s	Replicate s
Compression Pull-Off Test Assembly (CPOT)	Source 2 Pen 60/70	0.77	6.82	Cold	8
	Granite (40%) in Source 1 Pen 40/50	0.89	12.2	Cold	6
Direct Pull-Off Test Assembly	Source 2 Pen 60/70	0.71	16.78	Hot	6
	Gr (40%) in Source 1 Pen 40/50	1.01	33.86	Cold	6



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Figure 7: Correlational analysis of CPOT and DTT

447 **7 Comparison with pull-off stub and DTT approach**

448 A further qualitative comparison of CPOT with DTT approach of this study, pull-off stub as well
 449 as different direction tension approaches is presented in Table 7. The comparison ranging from
 450 specimen preparation and handling to examination of the failed surface. This comparison
 451 establishes the usefulness of this method for evaluating pull-off strength.

Table 7: Comparison of CPOT with Pull-off stub and DTT approach

Parameter	Pull-off stub and DTT approaches	Compression Pull-Off Test Assembly
Comparison with Pull Stub type tests	Pull-off stub type tests such as PATTI only measure maximum pull-off strength and do not taken into account load elongation behaviour.	Captures complete range of data required for testing of adhesive and cohesive bond.
Breakage of specimen during handling	In DTT approaches, specimen breakage is a common problem due to handling especially during clamping in UTM.	Out of dozens of specimens tested no specimen broke during fixing and handling.
Fitting into UTM	DTT tests requires careful clamping to avoid misalignment and breakage of specimen.	Fixing of specimen is done separately with improved mechanism. It does not require any attachments, the assembly is placed under loading shaft of UTM and contact is made manually.
Gripping of aggregate plates	The 3 point clamping mechanism explored were not good enough to account for variation in plate diameter. The second problem was gripping of in variations of dia.	Easier to account for larger variation in diameter as well as ability to accommodate imperfect circular plates.
Damage to Aluminium substrate	Increase chance of plate slip with screwing mechanism, also three-point clamping damaged the aluminium plates.	A thorough clamping mechanism did not damage any plate.

Load elongation curve adjustment	Curve correction is needed by plotting a tangent to the initial curve.	No curve correction needed, once manual seating is complete.
Cavitation	In pull stub tests it is difficult to fully analyse cavitation phenomenon. DTT of this study damaged some of the failure surfaces.	Cavitation was clearly captured during CPOT test and in the result plots.
Failed surface	The fail surface can easily get damaged during the testing in DTT devised in this research.	The plates are separated effectively there is no such issue
Limitations and further improvement	<ul style="list-style-type: none"> i. PATTI is designed to test at room temperature. Direct tension test requires UTM temperature control environment. ii. PATTI is reported to be reproducible and other methods require reproducibility. iii. Further studies and improved procedures are required in establishing film thickness. 	<ul style="list-style-type: none"> i. It is relatively easier to develop an integrated temperature control environment due to compression mechanism. ii. Reproducibility studies are required. iii. CPOT require further improved gap assembly and incorporation of other methods.

453

454 **8 Conclusions**

455 In this study, a novel test has been devised to evaluate cohesive and adhesive bond strength
 456 based on the principle of ‘tensile strength evaluation’. The research can be summarized in
 457 following conclusions:

458

- 459 • CPOT assembly is useful tool in evaluating bond strength (cohesive/adhesive). This
460 method provides a panacea to many problems faced in historical test methods (as
461 summarized in Table 9).
- 462 • The gap assembly is useful to achieve required film thickness under use in best
463 laboratory practices. This assembly need further modifications for practicality and
464 repeatability reasons.
- 465 • The test method devised in this research provides a useful insight into investigating
466 material behaviour.
- 467 • The material response in CPOT is more elaborative in terms of load elongation curve
468 than pull-off stub tests and direct tension test. Thus, this method makes it easier to
469 further understand the phenomenon of fibrillary nucleation in cohesive bond of
470 bitumen and mastics.

471

472 Finally, further research is required to evaluate special binders and adhesive properties. The
473 cases of moisture conditioning needs further evaluation. It will be advantageous to carryout
474 correlational analysis by use of other bond strength measurement techniques.

475

476 **Conflict of Interest**

477 None.

478

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486

487 **References**

488 Abd, Duraid M., Hussain Al-Khalid, and Riaz Akhtar. 2018. "Adhesion Properties of Warm-
489 Modified Bituminous Binders (WMBBs) Determined Using Pull-off Tests and Atomic
490 Force Microscopy." *Road Materials and Pavement Design* 19 (8): 1926–39.
491 <https://doi.org/10.1080/14680629.2017.1374993>.

492 Airey, Gordon D., and Young Kyu Choi. 2002. "State of the Art Report on Moisture Sensitivity
493 Test Methods for Bituminous Pavement Materials." *Road Materials and Pavement Design* 3
494 (4): 355–72. <https://doi.org/10.1080/14680629.2002.9689930>.

495 Al-Haddad, A.H.A.A., and H. Al-Khalid. 2015. "Development of a New Aggregate-Binder
496 Adhesion Test Method." *6th International Conference Bituminous Mixtures & Pavements*,
497 no. 2003: 385–94.

498 Alvarez, Alex E., Evelyn Ovalles, and Silvia Caro. 2012. "Assessment of the Effect of Mineral
499 Filler on Asphalt-Aggregate Interfaces Based on Thermodynamic Properties." *Construction
500 and Building Materials* 28 (1): 599–606. <https://doi.org/10.1016/j.conbuildmat.2011.08.089>.

501 Ameri, Mahmoud, Sareh Kouchaki, and Hossein Roshani. 2013. "Laboratory Evaluation of the
502 Effect of Nano-Organosilane Anti-Stripping Additive on the Moisture Susceptibility of
503 HMA Mixtures under Freeze-Thaw Cycles." *Construction and Building Materials* 48:
504 1009–16. <https://doi.org/10.1016/j.conbuildmat.2013.07.030>.

505 Apeageyi, Alex K., James R.A. Grenfell, and Gordon D. Airey. 2014. "Moisture-Induced
506 Strength Degradation of Aggregate-Asphalt Mastic Bonds." *Asphalt Paving Technology:
507 Association of Asphalt Paving Technologists-Proceedings of the Technical Sessions* 83

- 508 (January): 549–82. <https://doi.org/10.1080/14680629.2014.927951>.
- 509 Apeageyi, Alex K., James R.A. Grenfell, and Gordon D. Airey. 2015. “Influence of Aggregate
510 Absorption and Diffusion Properties on Moisture Damage in Asphalt Mixtures.” *Road
511 Materials and Pavement Design* 16 (March 2016): 404–22.
512 <https://doi.org/10.1080/14680629.2015.1030827>.
- 513 Bagampadde, Umaru, Ulf Isacson, and Badru M. Kiggundu. 2004. “Classical and
514 Contemporary Aspects of Stripping in Bituminous Mixes.” *Road Materials and Pavement
515 Design* 5 (1): 7–43. <https://doi.org/10.1080/14680629.2004.9689961>.
- 516 Baldi-Sevilla, Alejandra, Mavis L. Montero, José P. Aguiar-Moya, Luis G. Loria-Salazar, and
517 Amit Bhasin. 2017. “Influence of Bitumen and Aggregate Polarity on Interfacial Adhesion.”
518 *Road Materials and Pavement Design* 18 (March): 304–17.
519 <https://doi.org/10.1080/14680629.2017.1304265>.
- 520 Bhasin, Amit, Eyad Masad, Dallas Little, and Robert Lytton. 2006. “Limits on Adhesive Bond
521 Energy for Improved Resistance of Hot-Mix Asphalt to Moisture Damage.” *Transportation
522 Research Record* 1970 (1): 3–13. <https://doi.org/10.3141/1970-03>.
- 523 Blackman, Bamber R.K., Shuang Cui, Anthony J. Kinloch, and Ambrose C. Taylor. 2013. “The
524 Development of a Novel Test Method to Assess the Durability of Asphalt Road-Pavement
525 Materials.” *International Journal of Adhesion and Adhesives* 42: 1–10.
526 <https://doi.org/10.1016/j.ijadhadh.2012.10.013>.
- 527 Canestrari, Francesco, Fabrizio Cardone, Andrea Graziani, Felice Ausilio Santagata, and Hussain
528 U. Bahia. 2010. “Adhesive and Cohesive Properties of Asphalt-Aggregate Systems
529 Subjected to Moisture Damage.” *Road Materials and Pavement Design* 11 (October 2014):
530 11–32. <https://doi.org/10.1080/14680629.2010.9690325>.
- 531 Chang, W V. 1994. “Application of Acoustic Emission to Study the Cohesive and Adhesive

- 532 Strength of Asphalt.” *Strategic Highway Research Program SHRP-A-682*.
- 533 Chaturabong, Preeda, and Hussain U. Bahia. 2018. “Effect of Moisture on the Cohesion of
534 Asphalt Mastics and Bonding with Surface of Aggregates.” *Road Materials and Pavement
535 Design* 19 (3): 741–53. <https://doi.org/10.1080/14680629.2016.1267659>.
- 536 Cho, Dong-woo, and Hussain U. Bahia. 2010. “New Parameter to Evaluate Moisture Damage of
537 Asphalt-Aggregate Bond in Using Dynamic.” *Journal of Materials in Civil Engineering,
538 ASCE* 22 (3): 267–77. [https://doi.org/10.1061/\(ASCE\)0899-1561\(2010\)22:3\(267\)](https://doi.org/10.1061/(ASCE)0899-1561(2010)22:3(267)).
- 539 Copeland, A, J Youtcheff, N KRINGOS, A Scarpas, and S Mahadevan. 2006. “Determination of
540 Bond Strength as a Function of Moisture Content at the Aggregate-Mastic Interface.” *10TH
541 INTERNATIONAL CONFERENCE ON ASPHALT PAVEMENTS - AUGUST 12 TO 17,
542 2006, QUEBEC CITY, CANADA*, vol1,709-18.
- 543 Copeland, Audrey R. 2007. “INFLUENCE OF MOISTURE ON BOND STRENGTH OF
544 ASPHALT-AGGREGATE SYSTEMS.” Vanderbilt University.
545 <https://doi.org/10.1017/CBO9781107415324.004>.
- 546 Copeland, Audrey, Jack Youtcheff, and Aroon Shenoy. 2007. “Moisture Sensitivity of Modified
547 Asphalt Binders: Factors Influencing Bond Strength.” *Transportation Research Record:
548 Journal of the Transportation Research Board No. 1998* 1998: 18–28.
549 <https://doi.org/10.3141/1998-03>.
- 550 Fond, C. 2001. “Cavitation Criterion for Rubber Materials: A Review of Void-Growth Models.”
551 *J. Polym. Sci. B Polym. Phys.* 39 (17): 2081–96.
552 <https://doi.org/https://doi.org/10.1002/polb.1183>.
- 553 Harvey, J. A.F., and D. Cebon. 2003. “Failure Mechanisms in Viscoelastic Films.” *Journal of
554 Materials Science* 38 (5): 1021–32. <https://doi.org/10.1023/A:1022345713409>.

- 555 Harvey, J A F, and D Cebon. 2005. "Fracture Tests on Bitumen Films." *Journal of Materials in*
556 *Civil Engineering* 17 (1): 99–106. [https://doi.org/10.1061/\(ASCE\)0899-](https://doi.org/10.1061/(ASCE)0899-1561(2005)17:1(99))
557 [1561\(2005\)17:1\(99\)](https://doi.org/10.1061/(ASCE)0899-1561(2005)17:1(99)).
- 558 Howson, Jonathan Embrey. 2011. "Relationship Between Surface Free Energy and Total Work
559 of Fracture of Asphalt Binder and Asphalt Binder-Aggregate Interfaces." *PhD Dissertation,*
560 *The University of Texas at Austin*.
- 561 Huang, Shin-Che, Raymond E. Robertson, Jan F. Branthaver, and J. Claine Petersen. 2005.
562 "Impact of Lime Modification of Asphalt and Freeze–Thaw Cycling on the Asphalt–
563 Aggregate Interaction and Moisture Resistance to Moisture Damage." *Journal of Materials*
564 *in Civil Engineering* 17 (6): 711–18. [https://doi.org/10.1061/\(ASCE\)0899-](https://doi.org/10.1061/(ASCE)0899-1561(2005)17:6(711))
565 [1561\(2005\)17:6\(711\)](https://doi.org/10.1061/(ASCE)0899-1561(2005)17:6(711)).
- 566 Jakarni, Fauzan Mohd. 2012. "Adhesion of Asphalt Mixtures." *PhD Thesis , University of*
567 *Nottingham*.
- 568 Kanitpong, Kunnawee, and Hussain Bahia. 2005. "Relating Adhesion and Cohesion of Asphalts
569 to the Effect of Moisture on Laboratory Performance of Asphalt Mixtures." *Transportation*
570 *Research Record: Journal of the Transportation Research Board* 1901: 33–43.
571 <https://doi.org/10.3141/1901-05>.
- 572 Kiggundu, Badru M., and Freddy L. Roberts. 1988. "Stripping in HMA Mixtures: State-of-the-
573 Art and Critical Review of Test Methods." *NCAT Report 88-02, National Centre for*
574 *Asphalt Techonology, Auburn University, Alabama*.
575 <https://trid.trb.org/view.aspx?id=485075>.
- 576 Kim, Yong Rak, Ingrid Pinto, and Seong Wan Park. 2012. "Experimental Evaluation of Anti-
577 Stripping Additives in Bituminous Mixtures through Multiple Scale Laboratory Test
578 Results." *Construction and Building Materials* 29: 386–93.

- 579 <https://doi.org/10.1016/j.conbuildmat.2011.10.012>.
- 580 Kringos, N., Scarpas, A., and de Bondt, A. 2008. “Determination of Moisture Susceptibility of
581 Mastic-Stone Bond Strength and Comparison to Thermodynamical Properties.” In *Journal*
582 *of the Association of Asphalt Paving Technologists*, 77:435–78. Philadelphia.
- 583 Lottman, Robert P, R P Chen, K S Kumar, and L W Wolf. 1974. “A Laboratory Test System for
584 Prediction of Asphalt Concrete Moisture Damage.” In *53rd Annual Meeting of the Highway*
585 *Research Board Location: Washington District of Columbia, United States Date: 1974-1-21*
586 *to 1974-1-25*, 18–26.
- 587 Marek, C R, and M Herrin. 1968. “Tensile Behaviour and Failure Characteristics of Asphalt
588 Cements in Thin Films.” In *Assoc Asphalt Paving Technol Proc*, 2:pp 1-54.
589 <https://trid.trb.org/view/100898>.
- 590 Mohammed, Monketh, Tony Parry, Nick Thom, and James Grenfell. 2018. “Investigation into
591 the Bond Strength of Bitumen-Fibre Mastic.” *Construction and Building Materials* 190:
592 382–91. <https://doi.org/10.1016/j.conbuildmat.2018.09.084>.
- 593 Moraes, Raquel, Raul Velasquez, and Hussain Bahia. 2011. “Measuring the Effect of Moisture
594 on Asphalt-Aggregate Bond with the Bitumen Bond Strength Test.” *Transportation*
595 *Research Record: Journal of the Transportation Research Board* 2209: 70–81.
596 <https://doi.org/10.3141/2209-09>.
- 597 Peng, Chao, Pengxu Chen, Zhanping You, Songtao Lv, Ran Zhang, Fang Xu, Hao Zhang, and
598 Hanlin Chen. 2018. “Effect of Silane Coupling Agent on Improving the Adhesive Properties
599 between Asphalt Binder and Aggregates.” *Construction and Building Materials* 169: 591–
600 600. <https://doi.org/10.1016/j.conbuildmat.2018.02.186>.
- 601 Poulidakos, L. D., and M. N. Parti. 2011. “Micro Scale Tensile Behaviour of Thin Bitumen
602 Films.” *Proceedings of the Society for Experimental Mechanics, Inc.* 51 (7): 1171–83.

603 <https://doi.org/10.1007/s11340-010-9434-3x>.

604 Rahim, Abdur. 2017. "Effect of Fillers on the Oxidative Ageing of Bitumen and Cohesive Bond
605 Strength of Mastics." *PhD Dissertation, The University of Nottingham*.

606 Rice, J.M. 1958. "Elationship of Aggregate Characteristics to the Effect of Water on Bituminous
607 Paving Mixtures." *Symposium on Effect of Water on Bituminous Paving Mixtures, ASTM*
608 *STP No. 240*, 17–34. <https://doi.org/10.1520/STP39327S>.

609 Santagata, F A, F Cardone, F Canestrari, and Hussain U Bahia. 2009. "Modified PATTI Test for
610 the Characterization of Adhesion and Cohesion Properties of Asphalt Binders." *Sixth*
611 *International Conference on Maintenance and Rehabilitation of Pavements and*
612 *Technological Control (MAIREPAV6)Turin, Italy*.

613 Shah, Rachna, and Peter T Ward. 2003. "Lean Manufacturing : Context , Practice Bundles , and
614 Performance." *Journal of Operations Management* 21: 129–49.
615 [https://doi.org/10.1016/S0272-6963\(02\)00108-0](https://doi.org/10.1016/S0272-6963(02)00108-0).

616 Shute, John W., R.G. Hicks, James E. Wilson, and Lewis G. Scholl. 1989. "Effectiveness of
617 Antistripping Additives." *FHWA-OR-RD-89-03A*.

618 Solaimanian, Mansour, John Harvey, Maghsoud Tahmoressi, and Vivek Tandon. 2003. "Test
619 Methods to Predict Moisture Sensitivity of Hot-Mix Asphalt Pavements." *Moisture*
620 *Sensitivity of Asphalt Pavements: A National Seminar Topic 3*: 79–113.

621 Sultana, Sharmin. 2014. "Tensile Strength of Asphalt Binder and Influence of Chemical
622 Composition on Binder Rheology and Strength." *PhD Dissertation, The University of Texas*
623 *at Austin*. https://doi.org/10.2151/jmsj1965.74.5_639.

624 Taylor A, Mark, and N Khosla Paul. 1983. "Stripping of Asphalt Pavements: State of the Art
625 (Discussion, Closure)." *Transportation Research Record*, no. 911: 150–58.

626 <https://trid.trb.org/view/210085>.

627 Taylor, Publisher, Gholam Hossein Hamed, and Fereidoon Moghadas Nejad. 2014. “Road
628 Materials and Pavement Design Using Energy Parameters Based on the Surface Free
629 Energy Concept to Evaluate the Moisture Susceptibility of Hot Mix Asphalt” 16
630 (December): 37–41. <https://doi.org/10.1080/14680629.2014.990049>.

631 Terrel, Ronald L., and Saleh AI-Swailmi. 1994. “‘Water Sensitivity of Asphalt–aggregate Mixes:
632 Test Selection.’ SHRP-A-403 Washington, DC: Strategic Highway Research Program,
633 National Research Council. National Academy of Science.” [https://doi.org/ISBN:0-309-](https://doi.org/ISBN:0-309-05820-I)
634 05820-I.

635 Terrel, Ronald L, and J W Shute. 1989. “Summary Report on Water Sensitivity.”

636 Wang, Dongsheng, Junyan Yi, and Decheng Feng. 2014. “Modelling and Laboratory Studies on
637 the Adhesion Fatigue Performance for Thin-Film Asphalt and Aggregate System.”
638 *Scientific World Journal* 2014. <https://doi.org/10.1155/2014/819083>.

639 Zaidi, Syed Bilal Ahmed. 2018. “The Influence of Hydrated Lime on Moisture Susceptibility of
640 Asphalt Mixtures.” *PhD Dissertation, The University of Nottingham*.

641 Zhang, Jizhe, Gordon D. Airey, James Grenfell, and Alex K. Apeageyi. 2017. “Moisture
642 Damage Evaluation of Aggregate–bitumen Bonds with the Respect of Moisture Absorption,
643 Tensile Strength and Failure Surface.” *Road Materials and Pavement Design* 18 (4): 833–
644 48. <https://doi.org/10.1080/14680629.2017.1286441>.

645 Zhou, Lu, Weidong Huang, Feipeng Xiao, and Quan Lv. 2018. “Shear Adhesion Evaluation of
646 Various Modified Asphalt Binders by an Innovative Testing Method.” *Construction and*
647 *Building Materials* 183: 253–63. <https://doi.org/10.1016/j.conbuildmat.2018.06.064>.

648