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**A review on computer vision based defect detection and condition assessment  
of concrete and asphalt civil infrastructure**

Christian Koch<sup>a,\*</sup>, Kristina Georgieva<sup>a</sup>, Varun Kasireddy<sup>b</sup>, Burcu Akinci<sup>b</sup>,  
and Paul Fieguth<sup>c</sup>

<sup>a</sup> Chair of Computing in Engineering, Ruhr-Universität Bochum, Universitätsstraße 150, 44801  
Bochum, Germany;

<sup>b</sup> Dept. of Civil and Environmental Engineering, Carnegie Mellon University, Pittsburgh, PA  
15213, United States

<sup>c</sup> Dept. of Systems Design Engineering, Faculty of Engineering, University of Waterloo,  
Waterloo, Ontario, Canada N2L 3G1

\* Corresponding author: Phone: +49-234-32-26174; E-mail: koch@inf.bi.rub.de

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**Abstract:**

To ensure the safety and the serviceability of civil infrastructure it is essential to visually inspect and assess its physical and functional condition. This review paper presents the current state of practice of assessing the visual condition of vertical and horizontal civil infrastructure; in particular of reinforced concrete bridges, precast concrete tunnels, underground concrete pipes, and asphalt pavements. Since the rate of creation and deployment of computer vision methods for civil engineering applications has been exponentially increasing, the main part of the paper presents a comprehensive synthesis of the state of the art in computer vision based defect detection and condition assessment related to concrete and asphalt civil infrastructure. Finally, the current achievements and limitations of existing methods as well as open research challenges are outlined to assist both the civil engineering and the computer science research community in setting an agenda for future research.

**Keywords:**

Computer Vision, Infrastructure, Condition assessment, Defect detection, Infrastructure monitoring

**Research Highlights:**

- Visual inspection of civil infrastructure is essential for condition assessment.
- We focus on concrete bridges, tunnels, underground pipes, and asphalt pavements.
- Accordingly, we review the latest computer vision based defect detection methods.
- Using computer vision most relevant types of defects can be automatically detected.
- Automatic defect properties retrieval and assessment has not been achieved yet.

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4 **1. INTRODUCTION**  
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6 Manual visual inspection is currently the main form of assessing the physical and functional  
7 conditions of civil infrastructure at regular intervals in order to ensure the infrastructure still  
8 meets its expected service requirements. However, there are still a number of accidents that are  
9 related to insufficient inspection and condition assessment. For example, as a result of the  
10 collapse of the I-35W Highway Bridge in Minneapolis (Minnesota, USA) in 2007 13 people died,  
11 and 145 people were injured [1]. In the final accident report the National Transportation Safety  
12 Board identified major safety issues including, besides others, the lack of inspection guidance for  
13 conditions of gusset plate distortion; and inadequate use of technologies for accurately assessing  
14 the condition of gusset plates on deck truss bridges. A different, less tragic example is the  
15 accident of a freight train in the Rebunhama Tunnel in Japan in 1999 that resulted in people  
16 losing the trust in the safety and durability of tunnels. According to [2], the failure to detect shear  
17 cracks had resulted in five pieces of concrete blocks, as large as several tens of centimeters,  
18 which had fallen onto the track causing the train to derail.  
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20 In order to prevent these kinds of accidents it is essential to continuously inspect and assess the  
21 physical and functional condition of civil infrastructure to ensure its safety and serviceability.  
22 Typically, condition assessment procedures are performed manually by certified inspectors and/or  
23 structural engineers, either at regular intervals (routine inspection) or after disasters (post-disaster  
24 inspection). This process includes the detection of the defects and damage (cracking, spalling,  
25 defective joints, corrosion, potholes, etc.) existing on civil infrastructure elements, such as  
26 buildings, bridges, tunnels, pipes and roads, and the defects' magnitude (number, width, length,  
27 etc.). The visual inspection and assessment results help agencies to predict future conditions, to  
28 support investment planning, and to allocate limited maintenance and repair resources, and thus  
29 ensure the civil infrastructure still meets its service requirements.  
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31 This review paper starts with the description of the current practices of assessing the visual  
32 condition of vertical and horizontal civil infrastructure, in particular of reinforced concrete  
33 bridges (horizontal: decks, girders, vertical: columns), precast concrete tunnels (horizontal:  
34 segmental lining), underground concrete pipes (horizontal) (wastewater infrastructure), and  
35 asphalt pavements (horizontal). In order to motivate the potential of computer vision, this part  
36 focuses on answering the following questions: 1) what are the common visual defects that cause  
37 damage to civil infrastructure; 2) what are the typical manual procedures to detect those defects;  
38 3) what are the limitations of manual defect detection; 4) how are the defects measured; and 5)  
39 what tools and metrics are used to assess the condition of each infrastructure element.  
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41 Due to the availability of low cost, high quality and easy-to-use visual sensing technologies (e.g.  
42 digital cameras), the rate of creation and deployment of computer vision methods for civil  
43 engineering applications has been exponentially increasing over the last decade. Computer vision  
44 modules, for example, are becoming an integral component of modern Structural Health  
45 Monitoring (SHM) frameworks [3]. In this regards, the second and largest part of the paper  
46 presents a comprehensive synthesis of the state of the art in computer vision based defect  
47 detection and condition assessment of civil infrastructure. In this respect, this part explains and  
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4 tries to categorize several state-of-the-art computer vision methodologies, which are used to  
5 automate the process of defect and damage detection. Basically, these methods are built upon  
6 common image processing techniques, such as template matching, histogram transforms,  
7 background subtraction, filtering, edge and boundary detection, region growing, texture  
8 recognition, and so forth. It is shown, how these techniques have been used, tested and evaluated  
9 to identify different defect and damage patterns in remote and close-up images of concrete  
10 bridges, precast concrete tunnels, underground concrete pipes and asphalt pavements.

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14 The third part summarizes the current achievements and limitations of computer vision for  
15 infrastructure condition assessment. Based on that, open research challenges are outlined to assist  
16 both the civil engineering and the computer science research community in setting an agenda for  
17 future research.  
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## 22 **2. STATE OF PRACTICE IN VISUAL CONDITION ASSESSMENT**

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24 This section presents the state of practice in visual condition assessment of reinforced concrete  
25 bridges, precast concrete tunnels, underground concrete pipes and asphalt pavements.  
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### 28 **2.1 Reinforced concrete bridges**

29 As per US Federal Highway Administration (FHWA)'s recent bridge element inspection manual  
30 [4], during a routine inspection of a reinforced concrete (RC) bridge, it is mandatory to identify,  
31 measure (if necessary) and record information related to damage and defects, such as  
32 delamination/spall/patched area, exposed rebar, efflorescence/rust staining, cracking,  
33 abrasion/wear, distortion, settlement and scouring. While this list of defects comprises the overall  
34 list for common RC bridge element categories, such as decks and slabs, railings, superstructure,  
35 substructure, culverts and approach ways, not all defects are applicable to all components.  
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43 Table 1 highlights which defects are applicable to which components and hence need to be  
44 checked for each type of component on a bridge. While some of the stated defects are visually  
45 detected, some others of them may require physical measurements for accurate documentation  
46 and assessment. The size of the defect plays an important factor in deciding if it is necessary to go  
47 beyond the visual approach.  
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50 In addition to the list of defects stated above, FHWA also mandates that all bearings should be  
51 checked during inspection, irrespective of the material type and functional type of the bridge.  
52 Some of the relevant defects for bearings are corrosion, connection problems, excessive  
53 movement, misalignment, bulging, splitting and tearing, loss of bearing area, and damage.  
54 Furthermore, for seals and joints, inspectors focus on a specific set of defects, such as leakage,  
55 adhesion loss, seal damage, seal cracking, debris impaction, poor condition of adjacent deck, and  
56 metal deterioration or damage. While most of these defects can be detected visually, assessing  
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4 severity of the defects however needs close-up examination and measurements with suitable tools  
5 and equipment.  
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7 All of the existing defects on a bridge are categorized on a scale of 1 to 4 - each corresponding to  
8 the condition state of a particular element (1-Good, 2-Fair, 3-Poor, and 4-Severe). The condition  
9 state is an implicit function of severity and extent of a defect on a component. Though such  
10 categorization of condition states provides uniformity for each component and effects, the actual  
11 assessment that results in such categorization can be subjective. Table 2 provides some examples  
12 of guidelines provided in [4] for categorization of the condition states of different defects. Please  
13 refer to Appendix D2.3 in [4] for the complete list of guidelines for all defects.  
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16 There are typically three ways to perform manual inspection for concrete bridge elements: visual,  
17 physical and advanced. A combination of these methods is required depending on the condition  
18 of the bridge member under consideration. During visual inspection, an inspector focuses on  
19 surface deficiencies, such as cracking, spalling, rusting, distortion, misalignment of bearings and  
20 excessive deflection. Usually, the inspector can visually detect most of the relevant defects,  
21 provided there is suitable access to the bridge element. Visual inspections might not be adequate  
22 during the assessment of specific defect. For example, an inspector can identify visually that there  
23 is delamination when looking at a patch of concrete, but would not be able to gauge the extent  
24 and depth of it accurately by just visual inspection. Visual inspections, without utilization of any  
25 other inspection techniques, are also known to be subjective which might result in unreliable  
26 results [5] [6].  
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37 In contrast to the visual inspection, efforts during physical inspections are mainly towards  
38 quantifying the defects once they are identified visually. For example, to determine delamination  
39 areas in a pier or concrete deck, physical methods, namely, hammer sounding or chain drag may  
40 be used [7]. Measurements concerning expansion joint openings and bearing positions are also  
41 essential during the inspection and evaluation of a bridge. In some cases, advanced inspection  
42 methods like those based on strength, sonic, ultrasonic, magnetic, electrical, nuclear,  
43 thermography, radar and radiography, are used to detect sub-surface defects or for precise  
44 measurements of even surface defects.  
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## 50 **2.2 Precast concrete tunnels**

51 Precast concrete tunnels are one example of civil infrastructure components that are becoming  
52 increasingly important when developing modern traffic concepts worldwide. However, it is  
53 commonly known that numerous tunnels, for example in the US, are more than 50 years old and  
54 are beginning to show signs of deterioration, in particular due to water infiltration [8]. In order to  
55 support owners in operating, maintaining, inspecting and evaluating tunnels, the US Federal  
56 Highway Administration (FHWA), for example, has provided a Tunnel Operations, Maintenance,  
57 Inspection and Evaluation (TOMIE) Manual [8] and a Highway and Rail Transit Tunnel  
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4 Inspection Manual [9] that promote uniform and consistent guidelines. In addition, Best Practices  
5 documents summarize the similarities and differences of tunnel inspection procedures among  
6 different US federal states and transportation agencies [10].  
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10 There are different types of tunnel inspections: initial, routine, damage, in-depth and special  
11 inspections [8]. Routine inspections usually follow an initial inspection at a regular interval of  
12 five years for new tunnels and two years for older tunnels, depending on condition and age.  
13 According to [9], inspections should always be accomplished by a team of inspectors, consisting  
14 of registered professional engineers with expertise in civil/structural, mechanical, and electrical  
15 engineers, as both structural elements and functional systems have to be assessed. However, the  
16 focus of this review is on civil and structural condition assessment of precast concrete tunnels.  
17 Accessing the various structural elements for up-close visual inspection requires specific  
18 equipment and tools. Commonly, dedicated inspection vehicles, such as Aerial bucket trucks and  
19 rail-mounted vehicles, equipped with, for example, cameras (used for documentation), chipping  
20 hammers (used to sound concrete), crack comparator gauges (used to measure crack widths), and  
21 inspection forms (used to document stations, dates, liner types, defect locations and condition  
22 codes), are driven through the tunnel and permit the inspectors to gain an up-close, hands-on view  
23 of most of the structural elements.  
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31 More recently, integrated and vehicle-mounted scanning systems have entered the market. For  
32 example, the Pavemetrics Laser Tunnel Scanning System (LTSS) uses multiple high-speed laser  
33 scanners to acquire both 2D images and high-resolution 3D profiles of tunnel linings at a speed of  
34 20 km/h [11]. Once digitized the tunnel data can be viewed and analyzed offline by operators  
35 using multi-resolution 3D viewing and analysis software that allows for high-precision  
36 measurement of virtually any tunnel feature. A different system is the Dibat tunnel scanner that is  
37 manually moved through the tunnel [12]. It provides an actual comprehensive visual and  
38 geometrical image of the recorded tunnel surface. The corresponding tunnel scanner software  
39 allows easy, quick and versatile data evaluations to visualize the inspected tunnel and manually  
40 assess its condition.  
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47 According to [9], visual inspection must be made on all exposed surfaces of the structural  
48 (concrete) elements (e.g. precast segmental liners, placed concrete, slurry walls), and all noted  
49 defects have to be documented for location and measured to determine the scale of severity  
50 (Table 3).  
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57 Based on the amount, type, size, and location of defects found on the structural element as well as  
58 the extent to which the element retains its original structural capacity, elements are individually  
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4 rated using a numerical rating system of 0 to 9, 0 being the worst condition (critical, structure is  
5 closed and beyond repair) and 9 being the best condition (new construction) [9].  
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### 8 9 **2.3 Underground concrete pipes**

10 There is a great deal of buried infrastructure in modern cities, most of which appears to be out-of-  
11 sight and out-of-mind. Thus, whereas the number of cracks or depths of potholes in asphalt and  
12 concrete pavements may very well be the subject of water-cooler conversation, an interest in or  
13 an awareness of the state of underground sewage pipes is quite far removed from the perception  
14 of most citizens.  
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16 However there are two key attributes that motivate attention to underground infrastructure:  
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- 18 1. Being buried, the infrastructure is challenging to inspect
- 19 2. Being buried, the infrastructure is very expensive to fix or replace.  
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21 Indeed, the costs associated with sewage infrastructure modernization or replacement are  
22 staggering, with dollar figures quoted in the range of one or more trillion dollars [13].  
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24 There is, however, a strong incentive to undertake research and to develop sophisticated methods  
25 for underground concrete pipe inspection, due to the huge cost gap between trenchless approaches  
26 and the far more expensive digging up and replacement. The North American Society for  
27 Trenchless Technology and corresponding No-Dig conferences worldwide demonstrate the  
28 widespread interest in this strategy, dating back many years [14].  
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30 Direct human inspection, which is possible, at least in principle, for above-ground exposed  
31 infrastructure such as tunnels and road surfaces, is simply not possible for sewage pipes because  
32 of their relatively small size and buried state. Thus there has long been interest [15] in automated  
33 approaches, normally a small remotely-controlled vehicle with a camera.  
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35 A sewage pipe would normally be classified [16] into anticipated structures,  
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- 37 • Undamaged pipe
- 38 • Pipe joints (connections between pipe segments)
- 39 • Pipe laterals (connections to other pipes)

40 and some number of unanticipated problem classes:  
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- 42 • Cracks
- 43 • Mushroom cracks (networks of multiple, intersecting cracks, a precursor to collapse)
- 44 • Holes
- 45 • Damaged / eroded laterals or joints
- 46 • Root intrusion
- 47 • Pipe collapse

48 In common with other forms of infrastructure, the primary challenge to sewage pipe inspection is  
49 the tedium of manual examination of many hours of camera data, exacerbated by the sheer  
50 physical extent of the infrastructure which, in the case of sewer pipes, exceeds 200,000 km in  
51 each of the UK, Japan, Germany, and the US [17]. There are, however, a few attributes unique to  
52 sewage pipe inspection:  
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- Lighting is typically poor, since the only light available is that provided by the inspecting vehicle, and any forward-looking camera sees a well-lit pipe at the sides transitioning to completely dark ahead.
- Sewage pipes are subject to extensive staining and background patterning that can appear as very sudden changes in color or shade, giving the appearance of a crack.

Since the focus of this paper is on the computer vision analysis techniques, this following overview of data acquisition is brief, and the reader is referred to substantial review papers [15] [17] [18] [19]. Closed circuit television (CCTV) [15] [17] [20] [18] [19] [21] [22] [23] [24] is the most widespread approach to data collection for sewage pipe inspection; nevertheless the sewer infrastructure which has been imaged amounts only to a miniscule fraction of perhaps a few percent [19]. Because the most common approach is to have a forward-looking camera looking down the pipe, the CCTV method suffers from drawbacks of geometric distortion, a significant drawback in automated analysis.

Sewer scanner and evaluation technology (SSET) [15] [16] [25] [26] [19] represents a significant step above CCTV imaging. The pipe is scanned in a circular fashion, such that an image of a flattened pipe is produced with very few distortions and is uniformly illuminated. Laser profiling [17] [27] [28] [20] is similar to the SSET approach, in that a laser scans the pipe surface circularly, with an offset camera observing the laser spot and allowing the three-dimensional surface geometry of the pipe to be constructed via triangulation.

There are a few further strategies, albeit less common, for sewer pipe inspection. A SONAR approach [15] [28] [19] has been proposed for water-filled pipes, where most visual approaches will fail, particularly if the water is not clear. Ultrasound methods [29] [30] [31] [17], widely used to assess cracks in above-ground pipes, have been proposed to allow an assessment of crack depth, which is difficult to infer from visual images. Infrared Thermography [15] [17] [19] relies on the fact that holes, cracks, or water intrusion may affect the thermal behavior of the pipe and therefore be revealed as a thermal signature. Finally, ground penetrating radar [15] [17] allows the buried pipe to be studied from the surface, without the clutter and challenges of driving robots in buried pipes, but at a very significant reduction in resolution and contrast.

Because of rather substantial cost associated with data acquisition of sewer pipes, there is significant interest in maximizing the use of data. Prediction methods [32] [33] [34] [35] develop statistical, neural, or expert system deterioration models to predict pipe state, over time, on the basis of earlier observations.

## 2.4 Asphalt pavements

As reported by the American Society of Civil Engineers (ASCE), pavement defects, also known as pavement distress, cost US motorists \$67 billion a year for repairs [36]. Therefore, road surface should be evaluated and defects should be detected timely to ensure traffic safety. Condition assessment of asphalt pavements is essential to road maintenance.

There exist several techniques to detect distress in asphalt pavements. These techniques differ in the pavement data which is being collected and in the way this data is processed. Sensor-based

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4 techniques utilize devices to measure parameters of the pavement surface. Visual-based  
5 techniques make use of observations of the pavement surface to identify anomalies that indicate  
6 distress. Depending on the way of processing data, techniques are classified as purely manual,  
7 semi-automated or automated [37]. Manual processing is entirely performed by experts, while  
8 semi-automated and automated techniques require little or no human intervention.

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10 Visual-based techniques consist in manually inspecting the road surface or employing digital  
11 images and computing devices to assess the pavement condition. In case of manual inspection,  
12 trained personnel walks over the road shoulder and rates the pavement condition according to  
13 distress identification manuals. The disadvantage of this technique is that it is subjective despite  
14 the use of manuals and it depends on the experience of the personnel. Also, the personnel are  
15 exposed to traffic and weather, which makes the inspection procedure hazardous. Another issue  
16 related to the manual inspection of the road service is the time required to perform it.

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18 To speed up the assessment process, pavement images are analyzed instead of walking on the  
19 roads. Pavement images are obtained using downward-looking video cameras mounted on  
20 sophisticated vehicles. When the images and data are analyzed by human experts, the process of  
21 assessing the pavement condition is semi-automated. However, the rating of the pavement still  
22 depends on the experience of the analyzer and the subjectivity issue remains.

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24 Most distress detection techniques, regardless of whether they are manual, semi- automated or  
25 automated, depend on the pavement distress type. Pavement distress varies in its form and causes.  
26 Commonly, distress is characterized as alligator cracking, bleeding, block cracking, depression,  
27 longitudinal or transverse cracking, patches, potholes, rutting, raveling and more. The U.S. Army  
28 Corps of Engineers, for example, distinguishes between 19 types of distress [38].

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30 Distress types and measurements are defined in visual pavement distress identification manuals.  
31 Some of these measurements and indices vary between different countries, and federal states.  
32 Table 4 presents examples of defect assessment measurements and condition indices defined in  
33 such manuals [39] [40] [41] [42] [43]. As can be seen, severity and extent are present in most of  
34 the manuals. The common procedure to obtain the extent value is to count the occurrences of the  
35 different severity levels for each type of distress for the whole segment and convert the amount of  
36 distress into distress percentage.

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50 Condition assessment indices are calculated based on the distress measurements. Several  
51 pavement condition assessment indices have been developed and the procedures of their  
52 calculation are described in visual distress identification manuals. For instance, the pavement  
53 condition index (PCI) is widely used. The pavement condition index is a statistical measure of the  
54 pavement condition developed by the US Army Corps of Engineers [38]. It is a numerical value  
55 that ranges from 0 to 100, where 0 indicates the worst possible condition and 100 represents the  
56 best possible condition. A verbal description of the pavement condition can be defined depending  
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4 on the PCI value. This description is referred to as pavement condition rating (PCR). PCR  
5 classifies the pavement condition as failed, serious, very poor, poor, fair, satisfactory or good.  
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### 8 9 **3. COMPUTER VISION METHODS FOR DEFECT DETECTION AND ASSESSMENT**

10 This section presents a comprehensive synthesis of the state of the art in computer vision based  
11 defect detection and assessment of civil infrastructure. In this respect, this part explains and tries  
12 to categorize several state-of-the-art computer vision methodologies, which are used to automate  
13 the process of defect and damage detection as well as assessment. Figure 1 illustrates the general  
14 computer vision pipeline starting from low-level processing up to high-level processing (Fig. 1,  
15 top). Correspondingly, the bottom part of Figure 1 categorizes specific methods for the detection,  
16 classification and assessment of defects on civil infrastructure into pre-processing methods,  
17 feature-based methods, model-based methods, pattern-based methods, and 3D reconstruction.  
18 These methods, however, cannot be considered fully separately. Rather they build on top of each  
19 other. For example, extracted features are learned to support the classification process in pattern-  
20 based methods.  
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22 Subsequently, it is shown, how these methodologies have been used, tested and evaluated to  
23 identify different defect and damage patterns in remote and close-up images of concrete bridges,  
24 precast concrete tunnels, underground concrete pipes and asphalt pavements.  
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#### 36 **3.1 Reinforced concrete bridges**

37 Much of the research in defect detection and assessment using computer vision methods for RC  
38 bridges have largely focused on cracks, and to some extent on spalling/delamination and rusting.  
39 Many of these research studies targeted and contributed successfully to the automation of  
40 detection and measurement of defects. More studies need to be done to improve the methods  
41 used for automatic assessment as they are currently based on several assumptions.  
42

43 In addition to cracks, there are also other defects that are essential to be detected and assessed in  
44 relation to a RC bridge. Being able to detect, assess and document all defects as independent  
45 entities is paramount to provide a comprehensive approach for bridge inspection.  
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47 Currently, some of the other categories of defects are being inherently detected or assessed as part  
48 of other major dominating defects present at the using computer vision methods. For example,  
49 some methods detect abrasion as part of the crack [44]. In other cases, such as distortion and  
50 misalignment of bearings etc., no automated method exists for detecting and assessing them. This  
51 clearly indicates that more research needs to be done in the direction of automating the detection  
52 and assessment of various defects. To be able to perform automatic assessment and condition  
53 rating assignment, as a first step, it is necessary to identify the relevant defect parameters to  
54 accurately and comprehensively represent the defect information.  
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4 Below we will present the synthesis of the research done so far in the computer vision domain for  
5 various types of defects.  
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### 7 8 **3.1.1 Cracking** 9

10 Previously, Jahanshahi et al. [45] reviewed automatic defect detection approaches. Very recently,  
11 Rose et al. [46] reviewed existing crack detection and assessment algorithms for concrete bridges  
12 and classified them broadly as edge detection, segmentation and percolation, machine learning  
13 methods, morphology operations, ground and aerial robot photography, template matching, and  
14 other techniques. Building on this categorization, we reviewed and discussed some of the existing  
15 algorithms below.  
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18 Abdel-Qader et al. [47] compared various edge detection algorithms and found the Haar Wavelet  
19 method to be the most reliable among them, for the purpose of crack detection. However, the  
20 performance of edge detection algorithms on noisy image data is questionable, and same is the  
21 case with morphological operation based methods [48]. Yamaguchi et al. [49] used scalable local  
22 percolation-based image processing techniques and they proved to be efficient and accurate even  
23 for large surface images [50]. Abdel-Qader et al. [51] used a Principle Component Analysis based  
24 algorithm to detect cracks on a bridge surface. In this case, the accuracy of results varied with  
25 camera pose and distance from where images are taken. Prasanna et al. [52] developed a  
26 histogram-based classification algorithm and used it along with Support Vector Machines to  
27 detect cracks on a concrete deck surface. The results of this algorithm on real bridge data  
28 highlighted the need for improving the accuracy. Nevertheless, training data from various  
29 locations on the bridge could be used to build the classifier and testing could be done on data  
30 from a different location of similar structural composition. Similarly, Lattanzi and Miller [53]  
31 developed an automatic clustering method for segmentation based on Canny and K-Means to  
32 achieve greater accuracy of crack detection under various environmental conditions at a greater  
33 speed. Lattanzi and Miller's work is significant, especially if training data comprises images from  
34 different locations because it is important to offset the environment variability associated with  
35 variable lighting and shading conditions at different locations on the bridge, which is often the  
36 case with real world bridges. Some researchers also combined image-based 3D scene  
37 constructions with other techniques, in order to obtain depth perception that a 2D image lacks, to  
38 support automatic crack detection [54] [55].  
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41 While the above algorithms demonstrated capabilities to detect cracks, it is also important in a  
42 bridge inspection to understand the crack properties such as location, width, length and  
43 orientation, because condition ratings for bridge elements are assigned based on such properties.  
44 As outputs of the process of extracting properties from images are quantities, it is imperative that  
45 images are mapped to the global coordinate system. This requirement stems from the likelihood  
46 that images are collected on field with varying configurations i.e. resolutions, positions,  
47 orientations etc., over different inspections, which is primarily due to difficulty in replicating the  
48 same image capture configuration as well as a result of rapid advances in camera technologies  
49 over relatively shorter time periods. Towards normalizing different images to true world scale,  
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4 different researchers used techniques such as 3D pose estimation, multiple image stitching or by  
5 making measurements relative to the host structural element. In relation to that, some data  
6 acquisition systems used by researchers also had 3D pose control feature. These systems likely  
7 comprised surface-based (ground-based, water-based, bridge surface crawler) or aerial robots,  
8 which can either, have pre-configured settings or can log accurate image capture configuration  
9 dynamically.

10 Targeting to achieve the goal of going beyond mere crack detection, Yu et al. [56] developed a  
11 graph-based search method to extract crack properties for further assessment and used a ground-  
12 based robot for collecting images; however, this method needed manual input of start and end  
13 points of crack [50]. Later, Oh et al. [57] demonstrated a technique implementing automatic two-  
14 step: crack detection and crack tracing algorithm to be able to detect as well as identify crack  
15 properties, such as width and length, and tested the developed algorithm on a real bridge. They  
16 collected images with a ground-based robotic system that had controlled pan and tilt mechanisms,  
17 and used median filter for smoothening in the pre-processing stage, then isolated the candidate  
18 crack points and applied morphological operations such as dilation and thinning to maintain crack  
19 segment connectivity. As part of their study, they compared their results with Fujita, Sobel and  
20 Canny's method. The performance of the algorithm proposed by Oh et al. [57] matched the other  
21 three methods in terms of eliminating shaded regions and detecting major cracks, while  
22 outperforming them in the case of thinner cracks.

23 Other researchers targeted developing crack maps. Jahanshahi et al. [58] proposed a crack  
24 detection system to extract a complete crack map using 3D scene reconstruction, morphological  
25 operations and machine learning classifiers, and followed it up with a robust photogrammetry-  
26 based approach to compensate for camera perspective errors [59]. In another recent case, Zhu  
27 et al. [60] proposed a novel method involving thinning of the crack maps and subsequent  
28 measurement of each crack skeleton point to the crack boundary to automatically extract  
29 necessary crack parameters [50]. More recently, Lim et al. [61] proposed a Laplacian of Gaussian  
30 (LoG) based algorithm to perform crack detection and mapping on an RC bridge deck, and uses a  
31 mobile robotic system that can traverse a deck surface to capture images. The robot stores the  
32 spatial locations of image capture and uses robot coordinate system to transform from image  
33 coordinate system to global coordinate system.

34 The results presented in most of these cases were based on application of their methods on bridge  
35 deck surface, or in some cases image data of the beams and columns were considered. Generally  
36 speaking, most of the images used in these studies were images from simple flat and curved  
37 surfaces. However, the joints, seals, bearings and other connections present more complex  
38 geometry, often comprise of many sub-components and generally have varying material  
39 composition. Thus, these conditions render it hard to distinguish cracks from true edges. Also, as  
40 bridge inspectors commonly look out for connection related defects, algorithms should be tested  
41 on images from these components.

### 3.1.2 Delamination/ Spalling

Only recently, there have been developments in the detection and assessment of spalling on concrete surface and these works seem to have drawn inspiration from rusting detection and assessment [50]. German et al. [62] considered a combination of segmentation, template matching and morphological pre-processing, both for spall detection and assessment on concrete columns. They identified length of spalled region along longitudinal direction and distance between exposed reinforcement bars in the transverse direction and developed an approach for assessing the cumulative severity of the spalling based on different enumeration levels – (i) spalling of concrete cover without exposing reinforcement, (ii) spalling exposing longitudinal reinforcement and that of core concrete. The results obtained for the test images indicated spall detection with a precision of 81.1% and a recall of 80.2% for a set of 70 images. However, they indicated that more work is needed to achieve more detailed categorization of spall property result, with particular focus on spalling that exposes transverse reinforcement.

Adhikhari et al. [63] presented a novel approach based on orthogonal transformation, using shape preserving algorithms such as affine and projective transformation, to overcome perspective and parallax errors of a camera during data collection that can result in inaccurate defect quantification. They could determine if spalling had occurred, and if spalling was present, they could retrieve spall properties automatically. Their research also used Bridge Condition Index (BCI) after quantifying the defects to map them to condition ratings. While they could achieve reasonably accurate results (85% accuracy) for automatic procedures, their algorithm could not completely address automatic identification and assessment in situations where multiple defects (e.g. spall and crack) interact at the same spatial location.

Though work on spalling detection and assessment started only recently, the progress so far is very promising. The algorithms have been tested with images from decks and columns. Like in the case of cracks, even spalling needs to be checked for at concrete joints. Therefore, including images from those locations will be valuable for better detection and assessment performance of the algorithms.

### 3.1.3 Other damage scenarios

Zaurin et al. [3] used video imagery and bridge responses collected by strain gauges and fused them together to detect loss of connectivity between different composite sections, and change in boundary conditions. In the process, unit influence line of the bridge is extracted and statistical outlier detection is done to differentiate damage state from the baseline state. This method was tested using a four span experimental bridge belonging to University of Central Florida. Adhikari et al. [64] presented an change detection approach based on fourier transformation of the images, which could useful for detecting subtle defects such as periodic and sudden settlement of substructure. The review of the paper also suggests no proper basis for thresholding, and the results vary depending on the chosen threshold limit chosen. However, this method is a significant improvement over traditional change detection approach using the image difference, and can be used to quickly do a temporal comparison of different images. Uhl et al. [65]

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4 developed a method to detect deflection in structural members by applying homography mapping.  
5 Specifically, they implemented an automatic shape filter and a corner detector to calculate the  
6 deflection using homography mapping between the two views. They implemented this algorithm  
7 on an experimental set up in a lab, and also on a real bridge, and verified their results with the  
8 deflection calculated using a laser scanner. The results seem to be very accurate with the average  
9 difference between both the measurements being less than 0.5%. Though the deflection is being  
10 calculated accurately, it did not address the problem of damage localization and assessment.  
11 Kohut et al. [66] extended Uhl et al.'s work [65] to include damage localization and assessment  
12 using a wavelet transform based analysis method to do irregularity detection.  
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18 Various algorithms, related to detection and assessment of cracking, spalling and some damage  
19 scenarios in RC bridges, have been discussed above, and our focus was on the progress of the  
20 computer vision research in terms of automation in detection and assessment of these defects.  
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### 24 **3.2 Precast concrete tunnels**

25 In contrast to concrete bridge inspection, the image and video data acquired inside a tunnel is  
26 much different in terms of artificial lighting and camera distance. From that perspective, it is  
27 interesting to review the current state-of-the-art computer vision algorithms for defect detection in  
28 tunnel image data. According to Chaiyasarno [67], automated tunnel inspection systems that  
29 cover both defect detection and condition assessment can be grouped into the following themes:  
30 detection, visualization and interpretation.  
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#### 36 **3.2.1 Defect detection**

37 In analogy to concrete bridges, the most sought after defects are cracks as they are the primary  
38 indicator of deterioration patterns, which are due to other severe causes that need to be further  
39 analyzed [68]. Yu et al. [56] also highlight that cracks are of particular concern as they most  
40 significantly affect the state of the concrete within a tunneling environment.  
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42

43 Computer vision methods for crack detection generally involve a pre-processing step and a crack  
44 identification step. First, in the pre-processing step image processing techniques are applied to  
45 extract potential crack features, such as edges (threshold-based approaches). Second, the  
46 identification step usually applies crack modelling (model-based approaches) and/or pattern  
47 recognition techniques (pattern-based approaches) in order to classify if the extracted features  
48 belong to crack regions. Next to methods described in the previous section, mentionable  
49 contributions that are applicable to crack detection during tunnel inspection are the described  
50 below.  
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#### 56 **Threshold-based approaches**

57 Miyamoto et al. [69] calculate the difference in intensity between each pixel and the average  
58 intensity of each row in an image. A pixel that differs considerably from the average is said to be  
59 a crack pixel. Fujita et al. [70] use a line filter based on the Hessian matrix to emphasize line  
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4 structures associated with cracks before they apply thresholding to separate cracks from  
5 background.  
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7 The major drawback of threshold-based approaches is the question on how to choose a suitable  
8 threshold for extracting crack features. The described algorithms select a threshold based on prior  
9 knowledge. However, such methods can hardly be generalized and may be inapplicable to the  
10 imaging conditions found in real tunnel images. Moreover, they are prone to inaccuracy caused  
11 by shadows as the intensities of shadow pixels tend to have a similar brightness compared to  
12 crack pixels.  
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### 16 **Model-based approaches**

17 Ukai [71] developed a crack detection system based on the deformation of tunnel walls. Under  
18 this method, the model of a crack is characterized by eight quantities, such as area and Feret's  
19 occupancy rate. Subsequently, a filter is used to remove noise. Yamaguchi et al. [49] modelled  
20 cracks based on the concept of percolation, which is a physical model describing the phenomenon  
21 of liquid permeation. The algorithm starts by initializing a seed region and then the neighboring  
22 regions are labelled as crack regions based on the percolation process. Paar et al. [72] present a  
23 crack detection algorithm based on the line tracing algorithm that assumes a crack is a series of  
24 short straight lines connected together. Again, the algorithm starts from a seed point followed by  
25 searches for a line within the neighboring regions. Yu et al. [56] proposed a crack detection  
26 method in conjunction with a mobile robot system for automated inspection of concrete cracks in  
27 tunnels. Their method calculates the length, thickness and orientation of concrete cracks through  
28 a graph search; however, it requires the crack's start and end point to be manually provided.  
29 Moreover, the robot is required to maintain a constant distance from the tunnel wall in order to  
30 achieve accurate measurements of the damage properties. This system claims to have an overall  
31 detection accuracy rate of 75-85% and a measurement error of recognized cracks of less than  
32 10%.  
33

34 According to [67], model-based methods for crack detection strongly rely on user input to  
35 initialize the seed pixels. Consequently, hairline cracks may not be detection because users may  
36 be unable to identify the seed pixels. Due to reliance on the user input, these methods may not be  
37 scalable.  
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### 41 **Pattern-based approaches**

42 Liu et al. [73] apply a Support Vector Machine (SVM) classifier to determine if crack features  
43 appear in an image patch. Potential crack features are pre-defined based on intensity. Abdelqader  
44 et al. [51] use a Principal Component Principles (PCA) algorithm that reduces the dimensions of  
45 feature vectors based on eigenvalues, and extracts cracks from concrete images. The images are  
46 first pre-processed by line filters in three directions: vertical, horizontal and oblique; then further  
47 processed by the PCA algorithm and classified based on the nearest neighbor algorithm.  
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4 Methods based on pattern recognition considerably rely on training data in order to set up robust  
5 classifiers. Training and validation data are usually performed by manual labelling (supervised  
6 learning), which is a labor-intensive and error-prone procedure.  
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### 10 **3.2.2 Visualization**

11 The main goal of visualization is to visually organize large image and video data sets to enhance  
12 inspection. Image stitching or image mosaicing is a common method to combine and visualize a  
13 collection of images. In the domain of tunnel inspection, Chaiyasarn et al. [74] present a system  
14 that constructs a mosaic image of the tunnel surface with little distortion. Their system obtains a  
15 sparse 3D model of the tunnel by multi-view reconstruction [75]. Then, the Support Vector  
16 Machine (SVM) classifier is applied in order to separate image features lying on the cylindrical  
17 surface from those of the non-surface. The reconstructed 3D points are reprojected into images  
18 for accurate cylindrical surface estimation. Jahanshahi et al. [76] create stitched images of  
19 structural systems from a specialized camera that can tilt and pan. The method detects missing  
20 parts, such as bolts, when comparing images taken at different times for the purpose of structural  
21 health monitoring (SHM). The method applies a machine-vision algorithm to perform image  
22 registration to rectify images so that they are in the same coordinate frame.  
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28 In general, image stitching provides a feasible way of increasing the field of view that cannot be  
29 achieved by a single image. Consequently, a wide-angle or stitched image may improve defect  
30 detection results, in particular in case of hairline cracks, since the stitched image provides a  
31 higher resolution of defects, e.g. cracks.  
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### 35 **3.2.3 Change monitoring**

36 Apart from detecting cracks, classifying crack patterns and associated sizes, it is essential to  
37 observe if cracks in tunnel liners have changed over time and how quickly they do so. This kind  
38 of information helps determine the deterioration rate of the structural tunnel components [67].  
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41 Lim et al. [77] propose a system for change monitoring of cracks from multi-temporal images.  
42 Their system is based on a 2D projective transformation that can accurately determine the crack  
43 size, which is then monitored in consecutive images as the crack propagates. Although this  
44 system that can cope with images taken from different viewpoints, it requires explicit user input  
45 for the control points, which makes the system unscalable for a large number of images. Chen and  
46 Hutchinson [78] propose a framework for concrete surface crack monitoring and quantification.  
47 Their method is based on optical flow in order to track the movement of cracks. However, current  
48 solutions related to monitoring cracks or anomalies rely greatly on some degree of user input  
49 [67].  
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## 56 **3.3 Underground concrete pipes**

57 Deplorably, on the basis of a search of sewage pipe inspection methods currently offered by  
58 North American contractors, most buried pipe inspection continues to be manual and CCTV  
59 based, implying a slow inspection process subject to operator fatigue and boredom. Although this  
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4 limitation is frustrating, it strongly motivates continued research work on machine intelligence  
5 and computer vision in this application, and is the driving motivation for this section. There have  
6 been significant with computer vision contributions to pipe inspection, in whole integrated  
7 systems such as PIRAT [28] [18], KARO [18], and AIMP [18] [79], and the mapping the  
8 underworld (MTU) project [19].  
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11 The computer vision analysis of underground concrete sewer pipes has much in common with  
12 other forms of infrastructure. In particular, all of the parallel sections in this paper discuss aspects  
13 of crack detection, hole detection, and the classification of cracks into different forms or degrees  
14 of severity: multiple cracks, networked cracks etc. The forms of concrete deterioration in  
15 different parts of infrastructure do, after all, share a great deal in common.  
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18 As discussed in Section 2.3 and in review articles [15] [17] [18] [19], an unusually wide variety  
19 of possible imaging modalities has been developed for buried pipe inspection. In terms of the  
20 role of computer vision, we will focus our discussion on the most widespread methods, which  
21 have seen the most attention in the literature, namely the CCTV, SSET, and laser profiling  
22 methods. Other approaches, such as SONAR, ultrasonics, and ground penetrating radar do  
23 produce image-like data, but of a too specialized nature to consider here.  
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26 The analysis of buried sewage pipes possesses certain unique aspects which influence the  
27 associated computer vision strategy:  
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- 29  
30 • **Lighting:** The pipes are buried, dark and, depending on the modality of imaging, there  
31 may be constraints on the lighting possible, particularly in the case of CCTV imaging.
- 32  
33 • **Patterned background and contrast:** Sewage pipes suffer from significant degrees of  
34 deposits and staining, which may be dark, affecting image contrast, or may be highly and  
35 irregularly patterned, looking very much like any of a number of sewage failure classes –  
36 holes, single cracks, networks of cracks, root intrusion etc.
- 37  
38 • **Limited quality and quantity of data:** The slow, expensive approach to data collection  
39 strongly limits the total amount of data available for machine learning. Furthermore the  
40 lack of standardization – varied methodologies of imaging, machine standards, concrete  
41 pipe standards, concrete pipe contents and staining – make it challenging to learn broadly  
42 applicable approaches.  
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46 The methods of image analysis in the literature mostly involve feature extraction or modeling,  
47 both of which are widely used in computer vision and machine learning. Feature extraction [80]  
48 is the crucial bridge between a raw image and an information-rich feature vector that can be used  
49 for classification. The related problems of image modeling fall into three categories in the  
50 context of pipe inspection, from the most specific to the most abstract: of parametric / explicit  
51 models, morphology / shape-based models, and implicit / black-box models.  
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### 54 55 **3.3.1 Feature Extraction**

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57 Methods of pattern recognition and classification, such as a support vector machine or nearest  
58 neighbor classifier [80], expect to be given a vector of values describing the object to be  
59 classified. An image, containing thousands to millions of pixels, represents data in far too dilute  
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4 a form to be classified, since computation time and training data requirements are exponential in  
5 the number of dimensions. Feature extraction is essentially dimensionality reduction; in the  
6 context of analyzing images, computer vision has developed a vast range of approaches for  
7 extracting salient features.  
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10 Because buried concrete pipes are patterned and poorly lit, robust feature extraction is an  
11 essential step and appears throughout the pipe inspection literature. Methods include edge  
12 detection [25] [22] or the Hough transform [22] for edge/line detection, image segmentation [26]  
13 and background subtraction [18] for foreground object extraction, methods of image registration  
14 [18] and optical flow [24] for the tracking and association of objects in successive video frames,  
15 particularly relevant in CCTV imaging. More advanced methods include texture-based methods,  
16 including co-occurrence [21] and histograms of oriented gradients [23], and multi-resolution or  
17 wavelet-based approaches [29] [17]. Not all of these methods can be described here, and the  
18 reader is referred to a comprehensive review [81].  
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### 24 **3.3.2 Parametric models**

25 In principle, any object which we can recognize in an image, such as a crack, hole, or joint, can  
26 be modeled parametrically, with parameters explicitly describing properties such as width, length,  
27 radius, color etc. The strength of parametric models lies in their explicit nature, being relatively  
28 easy to understand and diagnose, however their limitation lies in their limited generalizability: in  
29 practice, any special case for which a given model is unprepared leads to a further iteration with a  
30 newly revised model addressing that case, and after repeated such iterations leading to ugly,  
31 clunky models containing a variety of exceptions.  
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34 Given an explicit model, the most fundamental, albeit slow, approach to detecting such objects in  
35 an image is using a generalized Hough transform [82] [83]. Essentially the Hough transform is a  
36 matched filter, placing the model in all possible parametric permutations at all points in the image  
37 and asking regarding degree of fit. If the number of parameters is sufficiently few, say two  
38 parameters describing the position plus one or two parameters describing size and shape, then the  
39 Hough approach may be possible, but given five or more parameters the Hough search space  
40 becomes far too large to search densely, and optimization approaches are needed.  
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43 Significant challenges for parametric approaches arise, by definition, for those objects which  
44 *cannot* be well modeled. So whereas a joint (line) or lateral (circle) is relatively simple, a crack is  
45 more challenging but may be modeled as a set of connected line segments, but a model to  
46 describe the wide range of appearances of root intrusions is very difficult. Most parametric  
47 computer vision models focus on crack detection, such as modeling a crack as being darker or  
48 having a higher variance than its immediate surroundings [25] or as a set of segments [22].  
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### 54 **3.3.3 Morphology**

55 Image morphology represents image shape on the basis of mathematical operations such as shape  
56 erosion (shrinking) and dilation (growing). The morphological approaches are more limited than  
57 parametric ones since, in principle, a parametric model can encode any imaginable behavior,  
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4 however the strength of morphological approaches is their elegance and operating in a manner  
5 similar to humans.  
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7 Any morphological operation is described or controlled through a structuring element, normally a  
8 relatively simple shape, such as a line, a rectangle, or a disc, which controls the extent to which a  
9 given pixel in the image affects its neighbors in dilating or eroding. Many textbooks and tutorial  
10 papers have been written [84] [85] and the interested reader is referred to them for greater  
11 background.  
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14 Much of pipe inspection is on the basis of binary (light/dark) primitive shapes, making image  
15 morphology a natural tool. The most basic shapes are elongated (cracks, joints) and round (holes,  
16 laterals), and so analysis can proceed on the basis of one or more round and one or more  
17 rectangular structuring elements. Recent uses of morphological approaches in buried pipes can  
18 be found in Sinha et al. [16], Su et al. [86], and Halfawy et al. [22].  
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### 23 **3.3.4 Neural models**

24 There has been a huge resurgence in computer vision interest in neural-like models, particularly  
25 in the area of deep belief networks [87]. The key advantage of a neural approach is that all stages  
26 of the problem – contrast enhancement, feature extraction, texture / shape analysis, classification  
27 – are machine learned all at once, in an integrated fashion. If the machine learning optimization  
28 converges well, then the integrated approach can offer robust classification.  
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31 On the other hand the sewage pipe problem, with huge numbers of images and a wide range of  
32 background patterning and texture, is a very large nonlinear optimization problem for which  
33 convergence may be poor. Neural-like methods are essentially black-box in nature, and therefore  
34 the actual effect or role of individual parameters is exceptionally hard to understand, in contrast to  
35 parametric models where the researcher can understand the operations of different parts of the  
36 algorithm and where, although parameters would ideally be machine learned, in principle the  
37 parameters could be tuned by hand on the basis of an understanding of their effect.  
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41 Nevertheless, the limitations of the preceding paragraph notwithstanding, neural approaches have  
42 seen rather significant application in buried pipe inspection. In most cases, the neural network is  
43 preceded by computer vision approaches for feature extraction, followed by neural learning [29]  
44 [27] [20] [21] or neuro-fuzzy approaches [26] [88].  
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### 48 **3.3.5 3D Reconstruction**

49 A final contribution from computer vision relates to the three dimensional reconstruction of a  
50 buried pipe, as a direct geometric detection of deep cracks and holes, rather than indirectly  
51 through visual appearance. The computer vision literature has developed a vast range of methods  
52 for 3D reconstruction, most notably shape from shading and stereo vision, both relatively  
53 complex problems. In contrast, the instruments for pipe inspection employ a laser and generate  
54 3D shape one dot at a time, a far more constrained problem and relatively simple compared to 3D  
55 scene reconstruction from images.  
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4 The use of laser reconstruction is widespread in computer vision, to generate 3D models of heads,  
5 limbs for prosthetics, or objects for 3D printing. For pipe inspection, methods for 3D  
6 reconstruction based on laser illumination are developed in Duran et al. [27] [20] and Kawasue et  
7 al. [89].  
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### 10 11 **3.4 Asphalt pavements**

#### 12 **3.4.1 Pre-processing**

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14 To automatically detect distress on pavement images, it is required to perform some  
15 preprocessing of the images. A common problem is that images are taken under different weather  
16 conditions or daytime and may contain shadows of trees. As a result non-uniform lighting is  
17 present in the images. Many of the methods for pavement distress detection are based on the  
18 assumption that distress pixels are darker than the background. Wang [90] and Tsai et al. [91]  
19 have concluded that such methods perform differently well according to varying lighting  
20 conditions and shadows. Figure 2 illustrates the so-called checker shadow illusion [92]. Square A  
21 looks darker than square B, but their pixel intensities are equal. This means, humans might be  
22 able to easily identify an asphalt crack in an image because it appears darker compared to the  
23 local background. Computers, however, may fail as they sometimes solely rely on global intensity  
24 values.  
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35 Several solutions to the non-uniform lighting problem have been proposed. Varadharajan et al.  
36 [93] select only images that were taken during daytime and when the weather was overcast or  
37 mostly cloudy, so that the lighting conditions are good. The disadvantage of this approach is that  
38 the selection process is also time-consuming and all captured images must be saved before  
39 selection and processing, which results in large amounts of data that is stored. Cheng [94]  
40 proposed a method to convert all images to a standardized background. For that purpose, a frame  
41 is split into rectangular windows. The average light intensity of the pixels in the windows is  
42 calculated for each window. Notably low average values are then replaced by the average value  
43 of the neighbor windows. Finally, multipliers are generated based on the average values. The  
44 multipliers are interpolated for each pixel so that all intensities vary around a base intensity. Zou  
45 [95] proposed a geodesic shadow-removal algorithm to remove the pavement shadows while  
46 preserving the cracks in images.  
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49 Another issue related to distress detection in pavement images is the presence of lane-marking on  
50 the images. Nguyen et al. [96] detect lane-marking regions and do not consider these regions for  
51 the distress detection. First, a binary image is obtained by applying a threshold. Second, the  
52 probabilistic Hough Transform is used to detect lines on this binary image. Lane-markings are  
53 detected based on the orientations and dimensions of these lines.  
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57 A range of techniques are applied to eliminate noise or for image enhancement. Lokeshwor [97]  
58 and Radopoulou [98] use median filtering and morphological operations (erosion, dilation,  
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4 opening, closing). Li [99] applies Gaussian smoothing for further denoising. Varadharajan [93]  
5 calculates the blur magnitude in the images and considers for assessment only images for which  
6 the blur-score is below a certain threshold. In some cases it might also be beneficial to compress  
7 the images to reduce the size and computation time, as done by Salman [100].  
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### 10 11 **3.4.2. Defect detection**

12 Several methods have been proposed, which are capable of detecting different types of distress in  
13 pavement images. Zhou et al. [101] use wavelet transform to decompose an image into  
14 approximation and detail coefficients. The detail coefficients represent distress in the pavement  
15 images. Zhou also proposed three statistical criteria and a norm of pavement distress  
16 quantification, which can be used as an index for pavement distress evaluation. Lokeshwor et al.  
17 [102] developed an algorithm which applies segmentation of distress pixels from the background  
18 pixels using an adaptive thresholding technique. User defined decision logic based on the area  
19 covered by the distress pixels categorizes video frames as frames with distress or frames without  
20 distress. Most detection methods are developed for a specific type of distress. Some of the  
21 methods are presented below.  
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#### 28 **Cracks**

29 As cracks are the most common distress type, a plenty of crack detection algorithms have been  
30 developed and presented. In particular, methods for real time crack analysis [103] [104], crack  
31 classification [105] crack depth estimation from vision [106], and automating crack sealing have  
32 been presented [107] [108].  
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35 Most of the algorithms for crack detection are based on the assumption that crack pixels are  
36 darker than the surroundings. Based on statistical measures of the pixel intensities, thresholding  
37 methods that classify pixels as crack or non-crack pixels are applied. Tsai et al. [91] have made a  
38 critical assessment of distress segmentation methods, in particular statistical thresholding, Canny  
39 edge detection, multiscale wavelets, crack seed verification, iterative clipping methods, and  
40 dynamic optimization based methods. Koutsopoulos et al. [109] developed an algorithm for crack  
41 image segmentation based on a model that describes the statistical properties of pavement images.  
42 Huang et al. [104] also proposed a classification method. An image is divided into cells.  
43 Depending on the contrast of each cell to its neighbor, the cells are classified as crack or non-  
44 crack cells. However, a limitation of the method is that it is hard to find a universal contrast  
45 threshold [91].  
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51 Salman et al. [100] proposed an algorithm which uses a Gabor filter. The preprocessed pavement  
52 image is convolved with the filter and the real component of the result image is thresholded to  
53 generate the binary image. Binary images resulting from differently oriented filters are combined  
54 and an output image is produced. The output image contains detected crack segments.  
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57 Moussa and Hussain [110] presented an approach for automatic crack detection, classification  
58 and parameter estimation based on machine learning. They apply Graph Cut segmentation to  
59 segment an image into crack and background pixels. A binary vector is created after  
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4 segmentation. Seven features are extracted from the vector for classification purposes. Then, a  
5 Support Vector Machine is used to classify the crack type in transverse cracking, longitudinal  
6 cracking, block cracking or alligator cracking. Moussa and Hussain also presented an approach to  
7 compute the crack extent and severity based on the length and the width of the crack in the image  
8 [110].  
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11 Varadharajan et al. [93] also use machine learning. They assume input images which can contain  
12 background, such as cars, traffic signs and buildings. First, the ground plane is segmented out  
13 from the rest of the image. After that, feature descriptors are computed based on the color and  
14 texture of the preprocessed pixels. A total of nine features and data obtained from human  
15 annotators are used to train a Support Vector Machine which classifies the images.  
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18 Li et al. [99] partition the image into crack regions and regions without cracks using the  
19 difference value between the maximum and the minimum grayscales of an image region. Then,  
20 the foreground is separated from the background by segmenting with Otsu's method and the  
21 images are classified using binary trees and back propagation neural networks.  
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24 Zou et al. [95] analyze the intensity difference in regions of the image to determine whether the  
25 pixels belong to cracks or not. After that, using tensor voting, a crack map is produced. In the  
26 crack map the probability of the pixels that are likely to be located along long crack curves is  
27 enhanced. The cracks in the image may sometimes be disconnected, so Zou et al. connect the  
28 crack parts with the help of an edge pruning algorithm.  
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### 32 **Potholes**

33 Usually, potholes also differ significantly from the background surface. Current computer vision  
34 research efforts in automating the detection of potholes can be divided into 3D reconstruction-  
35 based, 2D vision-based methods. Detection methods that are based on a 3D reconstruction of the  
36 pavement surface rely on 3D point clouds provided by stereo-vision algorithms using a pair of  
37 video cameras. Also there are hybrid systems available that use digital cameras to capture  
38 consecutive images of lines projected by infrared lasers [111]. A stereo-vision based surface  
39 model for comprehensive pavement conditioning has been proposed by Wang [112] and Hou et  
40 al. [113]. With the availability of a 3D point cloud, Chang et al. [114] have presented a clustering  
41 approach that can quantitate the severity and coverage of potholes and Jiaqiu et al. [115] have  
42 created a method for identifying, locating, classifying and measuring sag deformations like  
43 potholes and depression. The drawbacks of stereo-vision-based approaches are that they require a  
44 complete 3D reconstruction of the pavement surface and that the procedure of matching points  
45 between the two views is quite challenging due to the very irregular texture and color of the  
46 pavement surface.  
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54 Karuppuswamy et al. [116] integrated a vision and motion system to detect simulated potholes.  
55 Their approach detects potholes in the center of a lane. However, it relies on computer generated  
56 (simulated) potholes that are larger than 2 feet in diameter and white in color. The latter are  
57 simplified assumptions that do not reflect realistic pavement conditions. Jahanshahi et al. [117]  
58 used a depth sensor to detect and quantify defects in pavements. Based on the depth values of the  
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4 pixels, pixels are classified as deep or flat using thresholding. Then, the maximum depth of the  
5 defective regions is computed. However, the limitation of the proposed approach is that the data  
6 acquisition system, which is the Kinect sensor, is designed for indoor use. As a result, all the  
7 captured depth values are zero when the Kinect is exposed to direct sunlight.  
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10 Koch et al. [118] also presented a computer vision based approach for pothole detection in asphalt  
11 images. Based on surrounding shadows, elliptic shape and grain surface texture, the method  
12 identifies potholes in images. Image segmentation, shape approximation, and texture comparison  
13 are performed in this order. The image is divided into defect and non-defect pavement regions  
14 using histogram shape based thresholding and the triangle algorithm proposed by Zack et al.  
15 [119]. The shape of the pothole is approximated by applying morphological thinning and elliptic  
16 regression. Finally, the surface texture of the pothole candidate region is compared to the non-  
17 defect pavement region using spot filter responses. The region is determined as a pothole if the  
18 region inside the pothole candidate is coarser and grainier than the one outside. Koch et al.  
19 extended the method with video processing [120]. Using the described pothole detection method,  
20 potholes in a sequence of pavement images are counted.  
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### 27 **Patches**

28 Cafiso et al. [121] observed that pixels which belong to patches have different gray levels from  
29 the pixels which belong to the background. They use a clustering method to analyze the image  
30 with respect to patches.  
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33 Radopoulou et al. [98] detect patches in pavement images by applying morphological operations.  
34 Patch regions are segmented based on the assumption that patch pixels have greater intensities  
35 than pixels belonging to the background. Then, texture information is utilized and four different  
36 filters are applied. Subsequently, feature vectors of both intact and patch regions are constructed  
37 and compared after the convolution of the image with the filters.  
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## 41 **4. ACHIEVEMENTS AND CHALLENGES**

42 This section summarizes the current achievement and open challenges of computer vision for  
43 infrastructure condition assessment. A corresponding overview regarding the level of automation  
44 in defect detection and condition assessment is presented in table 5.  
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### 48 **4.1 Achievements**

49 When looking at defect detection and condition assessment of reinforced concrete bridges –  
50 classified as both vertical and horizontal civil infrastructure – it can be concluded that the current  
51 state-of-the-art computer vision based methods contribute successfully to the automation of  
52 detection and measurements of defects. The detection, localization and properties retrieval of both  
53 concrete cracks and concrete spalling is to a very large degree automated. Spalling defects can  
54 even be quantified and to some extent be mapped to condition ratings. Other important  
55 achievements include the ability of computer vision based methods to successfully support the  
56 detection of connectivity losses between composite sections, changes in boundary conditions,  
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4 changes in substructure settlements and deflection of structural members. The accuracy of vision  
5 based deflection detection can even compete with methods employing high accurate laser  
6 scanners.  
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9 With regard to very long horizontal civil infrastructure, such as precast concrete tunnels,  
10 underground concrete pipes and asphalt road networks, it is found that respective data collection  
11 technologies are fully automated. Moreover, available computer vision based algorithms  
12 successfully support the automation of detecting and localizing defects, such as cracks and joint  
13 spalling in concrete tunnels; cracks, holes and joint damage in concrete pipes; and cracks,  
14 potholes and patches in asphalt pavements. In case of bridge and tunnel inspection, computer  
15 vision based visualization methods (e.g. image stitching) successfully assist in defect detection  
16 and assessment as they improve the defect detection results due to better resolution. Concerning  
17 asphalt pavements, the crack properties retrieval procedure (type, with, length) is fully automated  
18 and some computer vision based distress quantification measures have the potential to be  
19 converted to indexes for distress assessment.  
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#### 25 **4.2 Challenges**

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27 Concerning computer vision supported concrete bridge inspection, it has to be mentioned that the  
28 process of image and video data collection is not yet fully automated. In terms of crack detection  
29 and assessment, existing methods need to be improved as performances on noisy data are  
30 questionable and accuracies vary with camera pose, camera distance and environmental  
31 conditions (lighting and shading at different locations). Moreover, several methods still require a  
32 significant amount of manual user input. In general, most of the methods assume images from  
33 simple flat and curved concrete surfaces, so that they may fail in cases of more complex  
34 geometries and material, such as joints, seals and bearings. Accordingly, there are currently no  
35 methods available that support the detection and assessment of bearing distortion and  
36 misalignment.  
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39 When looking at underground civil infrastructure, such as tunnels and pipes, it is concluded that  
40 poor lighting conditions, irregularly patterned background and contrast as well as limited data  
41 quality and quantity impose the most significant problems when dealing with computer vision  
42 based approaches to defect detection and assessment. With respect to lighting, common methods  
43 either use prior knowledge, thus can hardly be generalized or they rely on some degree of manual  
44 input and therefore do not scale well. More recent methods that use machine learning strongly  
45 rely on training data to create robust classifiers. Usually, the training process is based on  
46 supervised learning concepts (manual labeling) and is therefore labor-intensive and error prone.  
47 With regard to pipe inspection, the limited amount of data for machine learning and the lack of  
48 standardization on defect patterns prevent those methods to perform reasonably well. In addition,  
49 detection models with few parameters have limited generalizability, whereas models with many  
50 parameters fail in environments with a wide range of background pattern and texture due to the  
51 poor convergence of inherent non-linear optimization problems.  
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With respect to asphalt pavement monitoring, natural weather conditions and the daytime determine the success of available computer vision based defect detection and assessment methods. Shadows from trees, for example are very natural and prevent several methods, which usually work well in good lighting conditions, to perform reasonably well in real environments. Moreover, many algorithms endeavor to perform real-time and therefore are based on some kind of thresholding. However, these methods are not robust enough for image data with average image quality in practice as it is hard to find universal thresholds. Consequently, fully automated and comprehensive pavement distress detection and classification in a real-time environment has remained a challenge. Also, there is no comprehensive and robust method available to determine the severity level of distress for defect and condition assessment of asphalt pavements.

In general, reliable defect detection and condition assessment of civil infrastructure must be based not only on visual inspection methods. First, computer vision methods work under the principle “What you see is what you can analyze.” This means, that scenes under observation have to be sufficiently illuminated to make computer vision methods work. Visible shadows, for example, might have a significant impact on the capability of CV methods. In case of pothole detection shadows support the process, where in cases of 3D reconstruction they hinder the procedure. Moreover, the internal condition of infrastructure components cannot be captured, thus neither assessed using visual methods. On top of visual assessment techniques (whether manual or CV-supported), other advanced in-depth inspection methods (so-called Non-destructive evaluation (NDE) methods) are required to assess the overall condition, such as sonic, ultrasonic, magnetic, electrical, nuclear, thermography, radar technologies. However, defects on the surface are good indicators of the overall condition as they are part of many visual condition assessment manuals. Second, the data quality plays an important role in terms of noise, distance and perspective to the object of interest and the corresponding image resolution. For instance, if one wants to detect a crack of 1 millimeter width, he or she has to make sure that this 1 millimeter is mapped to a least 1 image pixel. Third, a number of safety risks are associated with working at certain heights and under heavy traffic. In this case, however, emerging remote-controlled unmanned aerial vehicles (UAV) might be a good practical solution for this issue. Forth, the operation of cameras always has to face privacy issues when monitoring public scenes, such as bridges and roads. Thus, it is recommended avoiding people in image and video data.

In summary, the authors conclude that more studies need to be conducted to improve the methods and algorithms for integrated condition assessment. It is currently not possible to detect, measure assess and document all different defects as independent entities to provide an integrated and comprehensive approach for bridge, tunnel, pipe and asphalt inspections. This is mainly due to the unsolved problem of identifying and assessing multiple interacting defects at the same location and the lack of standardization in identifying relevant defect parameters to comprehensively represent defect information. Moreover, no publically available large datasets exist to leverage supervised learning methods for the robust detection and classification of several infrastructure defect types.

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4 The following listing highlights the key research questions that have to be addressed by future  
5 research both in the civil engineering and computer science community in order to take the  
6 quality of computer vision based defect detection and condition assessment of civil infrastructure  
7 to the next level:  
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- 10 • How can we comprehensively detect, measure and assess interacting defect patterns at  
11 the same location to support integrated condition assessment of civil infrastructure,
- 12 • How can we generalize available detection models to adequately and universally address  
13 realistic environmental conditions, such as noisy image and video data, varying lighting  
14 conditions, different surface geometries and materials, and different camera poses and  
15 distances?
- 16 • How can we limit the amount of manual user input to improve the level of automation  
17 from poor defect detection to sophisticated defect and condition assessment?
- 18 • How can we create sufficiently large, publically available and standardized datasets to  
19 leverage the power of existing supervised machine learning methods for detection,  
20 classification and assessment of defects?
- 21 • How can we create unsupervised machine learning methods (online learning) for efficient  
22 training and on-demand updating of model parameters in defect detection and assessment  
23 models?  
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## 30 31 32 **5. SUMMARY**

33 To ensure the safety and serviceability of civil infrastructure it is essential to visually inspect and  
34 assess its physical and functional condition, either at regular intervals (routine inspection) or after  
35 disasters (post-disaster inspection). Typically, such condition assessment procedures are  
36 performed manually by certified inspectors and/or structural engineers. This process includes the  
37 detection of the defects and damage (cracking, spalling, defective joints, corrosion, potholes, etc.)  
38 existing on civil infrastructure elements, such as buildings, bridges, roads, pipes and tunnels, and  
39 the defects' magnitude (number, width, length, etc.). The condition assessment results are used to  
40 predict future conditions, to support investment planning, and to allocate limited maintenance and  
41 repair resources.  
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44 This paper has presented the current practices of assessing the visual condition of vertical and  
45 horizontal civil infrastructure, in particular of reinforced concrete bridges (horizontal: decks,  
46 girders, vertical: columns), precast concrete tunnels (horizontal: segmental lining), underground  
47 concrete pipes (horizontal) (wastewater infrastructure), and asphalt pavements (horizontal).  
48 Following this, the second and largest part of the paper has focused on a comprehensive synthesis  
49 of the state of the art in computer vision based defect detection and condition assessment of civil  
50 infrastructure. Several methodologies have been described and categorized, and literature on  
51 respective tests and evaluations on the current performances to detect and measure different  
52 defect and damage pattern in remote and close-up images of buildings, bridges, roads, pipes and  
53 tunnels has been presented. In the third part of this paper the current achievements and limitations  
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4 of computer vision for infrastructure condition assessment have been summarized. Finally, open  
5 research challenges have been outlined to assist both the civil engineering and the computer  
6 science research community in setting an agenda for future research.  
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**Table 1: Defects related to general bridge elements (Grey: Required; White: Not Required)[4]**

Element Name	Del/Spall	Exp Rebar	Eff/Rust	Crack	Abr/Wr	Distor	Settle	Scour	Damage
Deck									
Top Flange									
Slab									
Bridge Railing									
Closed Web/Box girder									
Girder Beam									
Stringer									
Arch									
Floor Beam									
Column									
Pier wall									
Abutment									
Pile cap/Footing									
Pile									
Pier cap									
Culvert									
Approach Slab									

\*Del/Spall- Delamination/Spall/Patched area; Exp Rebar- Exposed Rebar; Eff/Rust- Efflorescence/Rust Staining; Crack- Cracking; Abr/Wr- Abrasion/Wear; Distor- Distortion; Settle- Settlement; Scour- Scouring

**Table 2: Examples of defects and guidelines for assessment of condition states [1]**

Defects	Condition States			
	1	2	3	4
	Good	Fair	Poor	Severe
<b>Delamination/Spall/Patched Area</b>	None	Spall: < 1 inch depth or < 6 inch diameter	Spall: >1 inch & > 6 inch diameter; unsound patched area or if signs of distress	Situation worse than for Condition State 3 and if the inspector deems that it might affect the strength or serviceability of the element
<b>Efflorescence/Rust Staining</b>	None	Surface white without build-up or leaching without rust staining	Heavy build up with rust staining	Situation worse than for Condition State 3 and if the inspector deems that it might affect the strength or serviceability of the element
<b>Cracking</b>	Width < 0.012 inch or spacing > 3 ft	Width 0.012-0.05 inch or spacing 1-3 ft	Width > 0.05 inch or spacing < 1 ft	Situation worse than for Condition State 3 and if the inspector deems that it might affect the strength or serviceability of the element
<b>Abrasion/Wear</b>	No abrasion/wear	Abrasion or wearing has exposed coarse aggregate but the aggregate remains secure in the concrete	Coarse aggregate is loose or has popped out of the concrete matrix due to abrasion or wear	Situation worse than for Condition State 3 and if the inspector deems that it might affect the strength or serviceability of the element

**Table 3: Common civil/ structural defects of concrete tunnels and respective severity scales according to [9]**

Defect type / Severity	Minor	Moderate	Severe
Scaling	< 6 mm deep	6 – 25 mm deep	> 25 mm deep
Cracking	< 0.80 mm	0.80 – 3.20 mm, or < 0.10 mm (pre-stressed member)	> 3.20 mm, or > 0.10 mm (pre-stressed member)
Spalling / Joint Spall	< 12 mm deep or 75 – 150 mm in diameter	12 – 25 mm deep or ~150 mm in diameter	> 25 mm deep or > 150 mm in diameter
Pop-Outs (holes)	< 10 mm in diameter	10 – 50 mm in diameter	50 – 75 mm in diameter (> 75 mm are spalls)
Leakage	Wet surface, no drops	Active flow at volume < 30 drips per minute	Active flow at volume > 30 drips per minute

**Table 4: Examples of pavement defect assessment measurements and condition indices**

	Ohio [39]	British Columbia [40]	Washington [41]	South Africa [42]	Germany [43]
Measurement	Severity, extent	Severity, density	Severity, extent	Degree, extent	Extent
Index	Pavement condition rating	Pavement distress index	Pavement condition rating	Visual condition index	Substance value (surface)

**Table 5. Level of automation in computer vision based defect detection and condition assessment: (+) achieved, (~) partially achieved, (–) not achieved yet**

Infrastructure element	Defect type	Data collection	Defect detection		Defect properties retrieval (type, width, length, etc.)	Defect assessment	Condition assessment
			Presence	Location			
Reinforced concrete bridges	Cracks	~	+	+	+/~	~	–
	Spalling		+	+	+/~	~	
	Other: Loss of connectivity, substructure settlements, member deflection		~	~	~	–	
Precast concrete tunnels	Cracks	+	+	+	~	~	–
	Joint spalling		+	+	~	~	
Underground concrete pipes	Cracks	+	+	+	~	~	–
	Holes		+	+	~	–	
	Joint damage		+	+	~	–	
Asphalt pavements	Cracks	+	+	+	+	~	–
	Potholes		+	+	~	–	
	Patches		+	+	~	–	

\* Corresponding author: Phone: +49-234-32-26174; E-mail: koch@inf.bi.rub.de

\* Corresponding author: Phone: +49-234-32-26174; E-mail: [koch@inf.bi.rub.de](mailto:koch@inf.bi.rub.de)

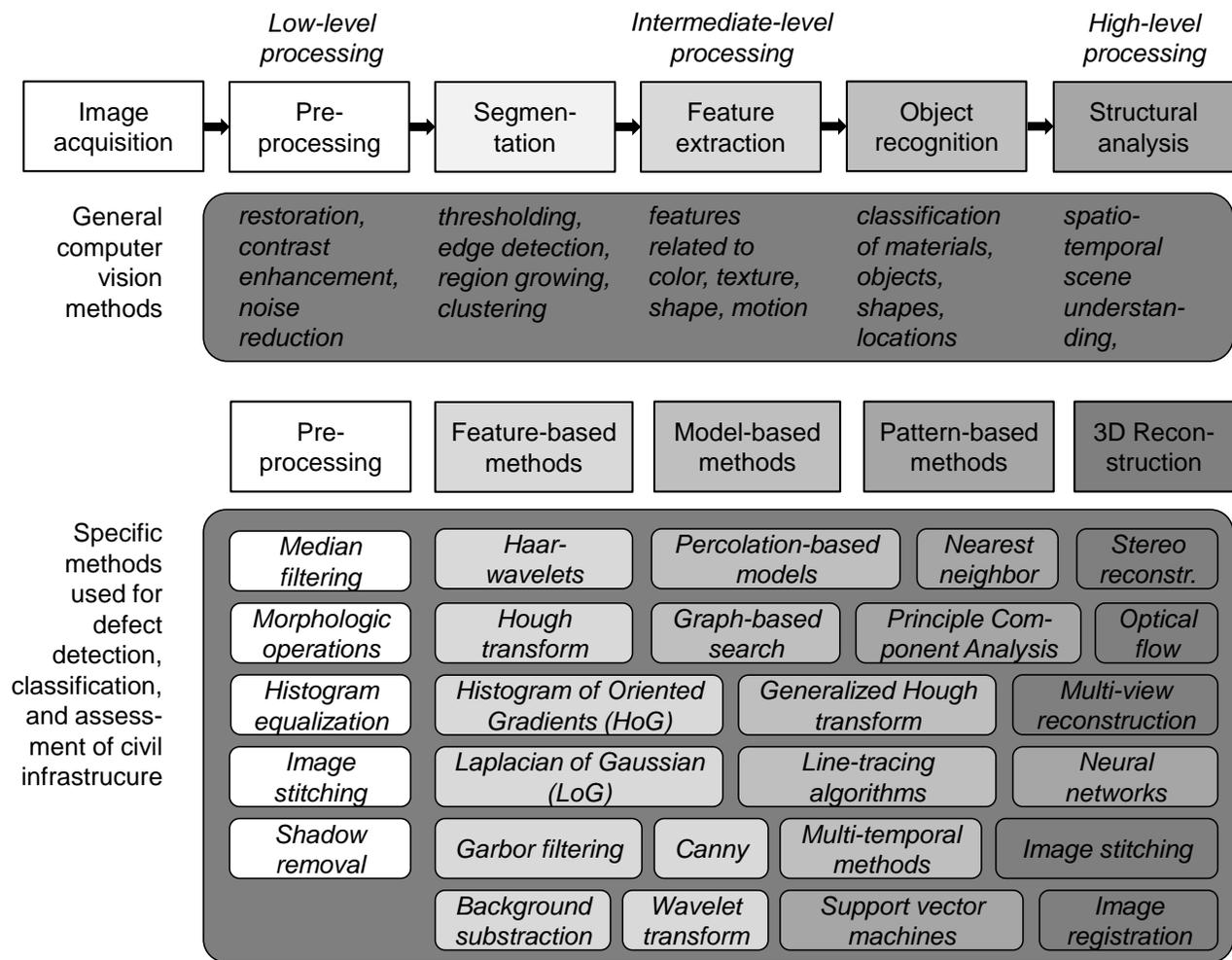
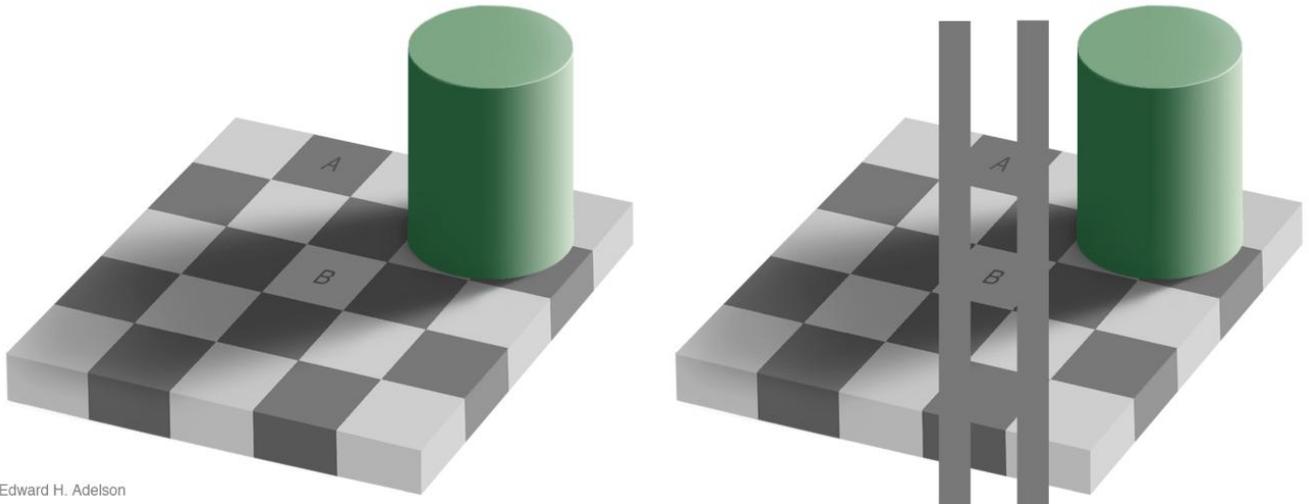


Figure 1: Categorizing general computer vision methods (top) and specific methods to defect detection, classification and assessment of civil infrastructure.



Edward H. Adelson

Figure 2: The checker shadow illusion [88]: The squares marked A and B share the same grey intensity (©1995, Edward H. Adelson).